STORMWATER MANAGEMENT REPORT

Proposed Warehouse Facility 10 Russell Road East Granby, CT

Prepared For:

Joneser's Express Transportation (aka JET)
Windsor Locks, CT

Prepared By:

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FAHA Project #23145

September 29, 2023 Revised Through December 22, 2023



1. Introduction

This Stormwater Management Report has been prepared to demonstrate that the stormwater management practices for the proposed development follow sound engineering practices, adhere to the Town of East Granby Stormwater Management Plan and Town Engineering Guidelines, CT DEEP Stormwater Treatment Practices for capture and treatment of the minimum recommended water quality volume and protect adjacent landowners and the downstream watershed from adverse stormwater impacts. The East Granby Stormwater Management Plan incorporates the requirement to implement Low Impact Development (LID) practices. Several of the recommended practices have been incorporated in this design.

This report presents hydrologic analysis of both pre- and post-developed conditions to demonstrate that the development of the parcel will result in no net increase in peak rates of runoff or total volume of runoff from the development for storms of return periods of 2, 5, 10, 25, 50 and 100 years.

This report also presents a pipe-to-pipe design analysis to demonstrate that all proposed storm-drain systems have adequate capacity to convey runoff from the site for a 25-year return period storm event.

2. Project Description

The proposed development site is comprised of three parcels totaling 12.44-acres located on southeasterly side of the intersection of Russell Road and East Street and is located in the CP-T Zone. Except for an existing single-family house and garage, the site is currently an open undeveloped mowed meadow grass lot. The site had been used for many years as a seasonal weekly classic car auto show ground. A combination of open space, residential, industrial, warehouses and commercial uses surround the site. See attached existing topographic conditions survey Sheet ILP-1.

The proposal includes construction of a single-story warehouse facility consisting of one 40,000 SF building with truck loading docks and truck and employee vehicle parking areas. Driveway access will be from two new paved drives off Russell Road. Utility service lines will then be extended underground to the northerly side of the new building from existing public utility services in Russell Road. The site will have private well, fire water storage tank and a conventional septic system.

Storm water management systems proposed for the development will consist of storm drain systems comprised of roof drains, catch basins and culverts and three excavated Water Quality/Detention Basins. Stormwater runoff from the roof, all on-site paved drives and parking area will be directed to the basins where stormwater will be captured, detained/retained, and treated before discharge. Catch basins will have 2-ft. sumps and outlets will be hooded prior to discharge to the basins.

Outflow from the three basins will be controlled by an individual concrete outlet structure with a controlling concrete orifice and overflow weir. Rip rap protection designed using CTDOT standards is proposed at all outfalls to control downgradient erosion.

For the overall drainage analysis presented, two Design Points (East and South) serve as the focal point of the overall site hydrologic analysis, as discussed below and shown on the attached Sheets DA-1 and DA-2.

3. <u>Hydrologic Analysis</u>

A comprehensive hydrologic analysis was conducted for both the existing condition and the proposed developed condition of the site to determine peak flow of runoff under both conditions. Hydraflow Hydrographs computer software was utilized in the runoff analysis. The SCS Method was used to model peak flows for pre- and post-redeveloped conditions including the sizing for the three basins.

For purposes of the hydrologic analysis, it is assumed that all site runoff, under both existing and proposed conditions flows to the two Design Points as shown on Sheets DA-1 and DA-2. Watershed maps used in the analysis are attached.

Approximately 2/3's of the development site currently drains overland southerly towards the flagged wetlands and unnamed intermittent watercourse that follows along the southerly property line as shown on Sheet DA-1. The watercourse which flows to the east is abutting Town Open Space. The 1/3 portion located in the northeasterly portion of the site drains overland to the abutting parcel and is then directed to the south and east to join up with flow coming from the intermittent watercourse. This receiving wetland and watercourse are all part of the overall watershed associated with DeGrayes Brook. DeGrayes Brook flows north and east of the site crossing under Russell Road approximately 750 feet to the east of the project site. The brook is one of the smaller tributaries making up the much larger watershed of Stony Brook. See the attached town watershed map.

In the existing conditions analysis, the time of concentration (Tc) was determined using TR-55 methodologies for the aggregate of flow components consisting of overland sheet flow and shallow concentrated flow as shown on Sheet DA-1. Based on the NRCS Soil Survey, the upland soils within the proposed developed portion of the site are made up of moderate to well-draining silt loams typically found in the surrounding former farmlands. The soil type 25B-Brancroft silt loam falls in Hydrologic Group C. The soil type 82B/C-Broadbrook silt loam falls in the Hydrologic Group C as well. Portions of the developed area located in the southwesterly portion of the site fall into a mixture of the soil types 5-Wilbraham silt loam and 9-Scitico, Shaker and Maybid soil which are poorly drained and tend to be siltier with some clay. See Attachment B.

In the proposed analysis, the site is divided into sub-catchment areas in the developed portion of the parcel represented by relatively small watersheds as shown on Sheets DA-2. A starting Tc of 5 minutes was used for pavement and roof surfaces with an average travel time of 2 additional minutes added resulting in a final design Tc of 7 min. for pipe sizing.

SCS Runoff Curves Numbers were based on the following values. Detailed calculation sheets for each of the sub-catchment areas are attached.

In general:

- For impervious areas (i.e. rooftops and paved areas), CN=98 was used.
- For open meadow grass land, CN=58 to 71 was used.
- For moderately sloped wooded portions of the site, CN=77 was used.

Analysis was performed for the 2-, 5-, 10-, 25-, 50-, and 100-year return period storm events using the 24-hour duration storm event for East Granby using the NOAA Atlas 14 data, which is required by current CT DOT Drainage Manual design standards. See Attachment C.

The hydrologic analysis presents a comparison of pre-and post-developed conditions for each of the two design points East and South.

Existing Conditions Analysis

The existing conditions watershed area was utilized to model existing site conditions and to develop a baseline for comparison. The two delineated areas represent the total contributory watersheds that flow to the two points of analysis which are ultimately captured and routed through water quality/detention basin systems in the post developed model. The limits of the watersheds were determined by reviewing on-site topography from the existing conditions survey and field inspection. The limits of the modeled two sub-watersheds are depicted on Sheet DA-1.

The existing conditions watershed consists of essentially the entire property down to the south property line and over to the easterly boundary line. Final times of concentration (Tc) ranging between 7.5 and 9.1 minutes were calculated for the existing conditions subwatersheds. Peak rates of flow for the sub-watersheds and the overall Existing Conditions Watershed for the various modeled storms (2 to 100-year return period events) are summarized in Table 1 and included in Attachment D.

Proposed Conditions Analysis

For proposed conditions, the sub-catchment areas were utilized to model proposed site conditions. For comparison, the total area modeled under the proposed site conditions is essentially equal to that modeled under the existing site conditions. There are a total of two discharge points from the proposed basins following the general patterns of runoff from the site to the downstream wetlands and off-site intermittent watercourse under current conditions.

The limits of the modeled watersheds, as well as the locations of the three-water quality/detention basins are depicted on Sheet DA-2. To remain conservative in the analysis, the beneficial impacts of flow and volume reduction through possible infiltration and inflow within the basins are not included in the proposed hydrologic modeling. Because an

increase in impervious area results in increases of peak rates of runoff, storm water detention is proposed. The water quality/detention basins will mitigate increases in peak rates of runoff through use of containment and outlet methods designed to throttle back peak rates of flow.

The hydraulic model combines the overflow outflow from the two discharge points and adds to the flow from the un-detained watershed for a total proposed conditions peak rate of flow. The peak rates of discharge for the existing and proposed site conditions for the modeled watersheds for the 2-, 5-, 10-, 25-, 50- and 100-year storm events were computed by the program. The design goal was to be below pre-development flow conditions at each of the two design points. Results of the combined analysis are presented below in Table 1.

TABLE 1

Design Point East	Existing Conditions	Proposed Conditions Peak Rate of Discharge (CFS)		
Return Period (years)	Peak Rate of Discharge (CFS)			
2-Year	3.13	0.63		
5-Year	5.59	1.07		
10-Year	7.70	1.44		
25-Year	10.94	2.00		
50-Year	13.56	2.44		
100-Year	15.96	2.85		

Design Point South	Existing Conditions	Proposed Conditions		
Return Period (years)	Peak Rate of Discharge (CFS)	Peak Rate of Discharge (CFS)		
2-Year	10.29	7.69		
5-Year	18.43	12.43		
10-Year	25.42	16.90		
25-Year	36.18	23.06		
50-Year	44.85	27.62		
100-Year	52.82	31.68		

The model indicates that the stormwater management methods proposed will reduce peak flows in each sub-watershed and overall, when combined flow is compared at the design points. Details of the analysis of the Hydraflow model, including model input and output are included in Attachment D.

4. Pipe to Pipe Design Analysis

The proposed development will employ conventional on-site storm drain systems. These systems convey runoff to the water quality/detention basins for peak flow attenuation and water quality purposes. A detailed, pipe-to-pipe analysis was conducted for all of the proposed on-site storm drain systems. All roof leaders will be piped to the systems as well

The storm drain systems have been designed to handle the peak flow for the 25-year storm event, in accordance with town requirements. To design and analyze the pipe systems, a pipe-to-pipe analysis was conducted using Hydraflow Storm Sewers for Windows software. This software uses the Rational Method and Manning's Formula to compute peak flow to each basin, and to calculate the capacity of individual culverts.

Input data includes the geometry and configuration of the storm drain system, catchment area of each inlet, weighted runoff coefficients, and times to inlet. Catchment areas were calculated based on proposed topography utilizing polyline delineations in AutoCAD. The catchment areas are depicted graphically on Sheet DA-3.

Weighted runoff coefficients were calculated based on percentages of impervious and pervious areas within each catchment area, as determined by areas of pavement, rooftops, landscaped and wooded areas. The following runoff coefficients were used in the post-development conditions hydrologic model: For impervious areas, C=0.9 was used. For landscaped areas, C=0.30 was used.

Rainfall intensity data was taken from NOAA Atlas 14 rates off of the NOAA website for East Granby. A copy of the Rainfall Intensity Curve is presented in Attachment C. All inlets are small and highly urbanized, therefore, the time to inlet for all inlets is assumed to be five minutes. Manning's roughness coefficient of 0.013 was used for specified N-12 CPE and RCP pipe.

The model calculates the capacity of each culvert and accounts for loss coefficients at bends and inlet and outlet control, whichever governs. Input data includes basin geometry, longitudinal slope, cross slope, and basin depression. Standard size CT DOT 'Type-C' and Double catch basins were modeled for all basins, as appropriate.

Results of analysis are attached and include summaries of system design based on CT DOT output formats. Program input and output data reports are presented in Attachment E. The analysis indicates that all storm drain culverts are designed to adequately convey the 25-year storm event.

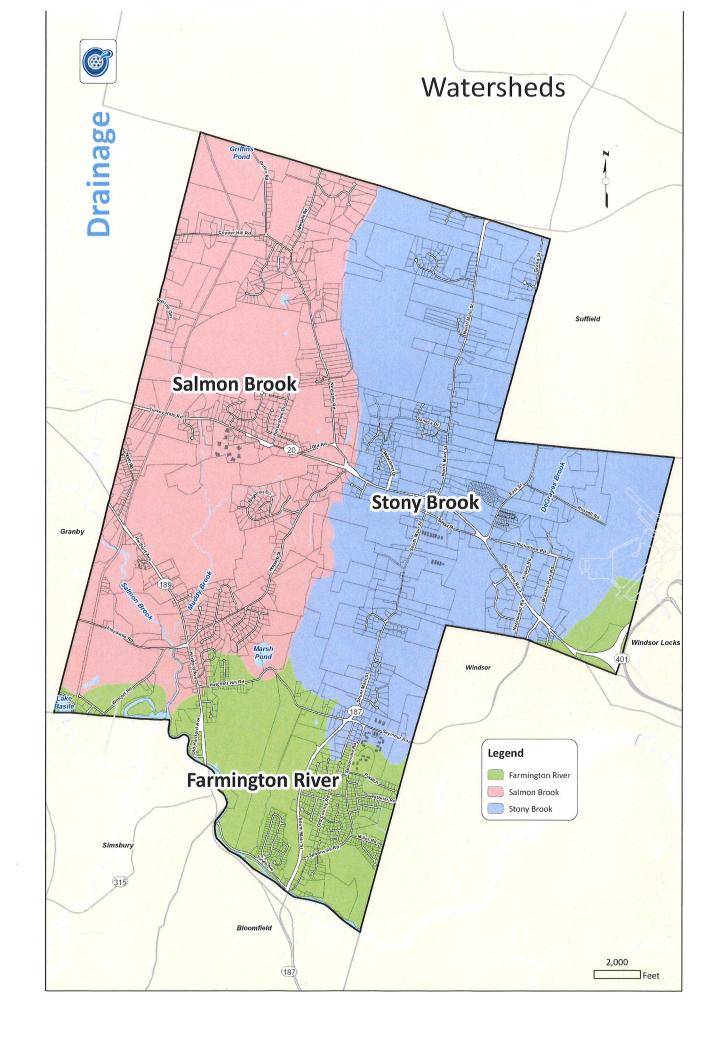
5. Water Quality Treatment Practices

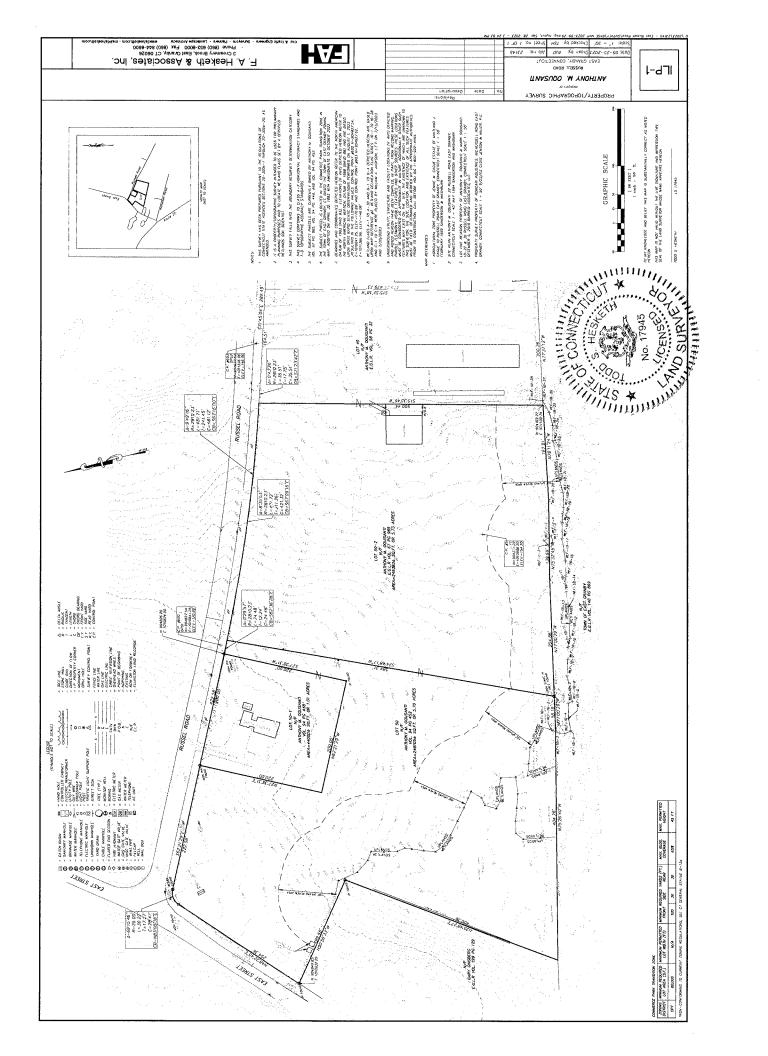
The treatment practice employed for the paved surfaces consists of a treatment train starting with catch basins with 2-foot sumps, hooded outlets, discharge to water quality /detention basins, and then discharge to riprap outlet aprons sized per CTDOT design standards.

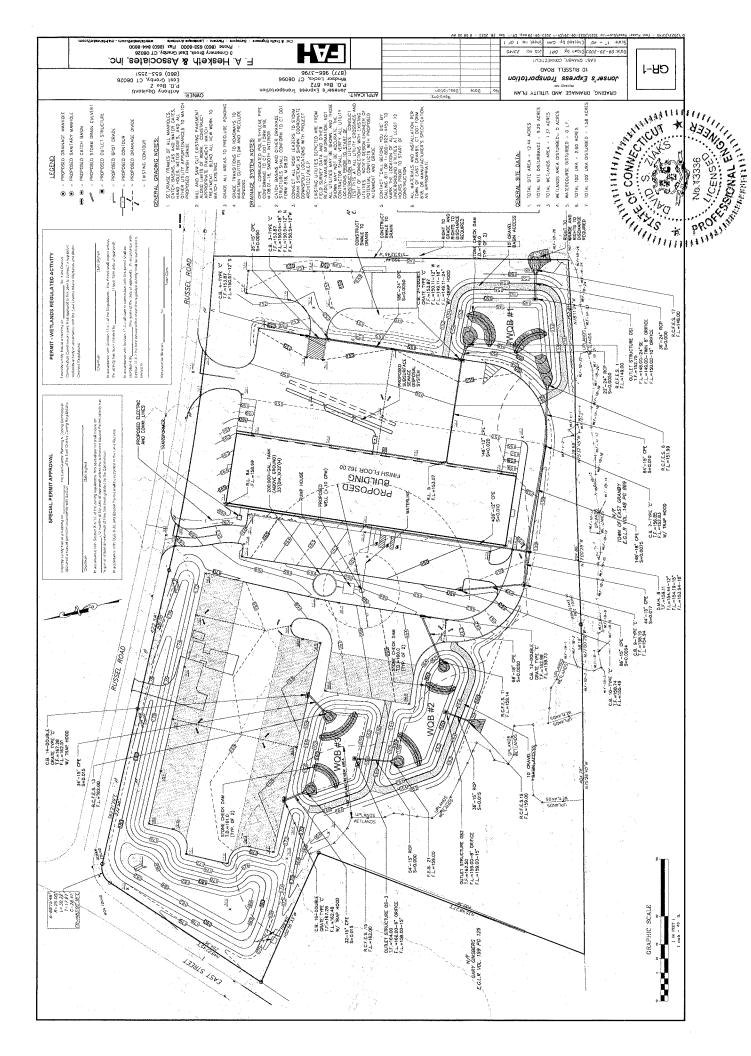
Water Quality volume (WQV), Sediment Forbay, and Rip Rap Level spreader calculations were completed following the Stormwater Treatment Practice Sizing Criteria per Chapter 13 of the 2023 Connecticut Stormwater Quality Manual (SWQM). The three-water quality/detention basins are proposed as an integral part of the stormwater management of the site runoff. The water quality/detention basins are designed to capture and treat more than the minimum required Water Quality Volume (WQV) recommended by the 2023 Connecticut Stormwater Manual (SWQM). The WQV calculations are included in Attachment F.

6. Summary

Both the hydrologic and pipe-to-pipe analyses demonstrate that the goals of the design effort have been met. More than the minimum recommended WQV is provided and peak flows have been reduced below peak existing conditions.

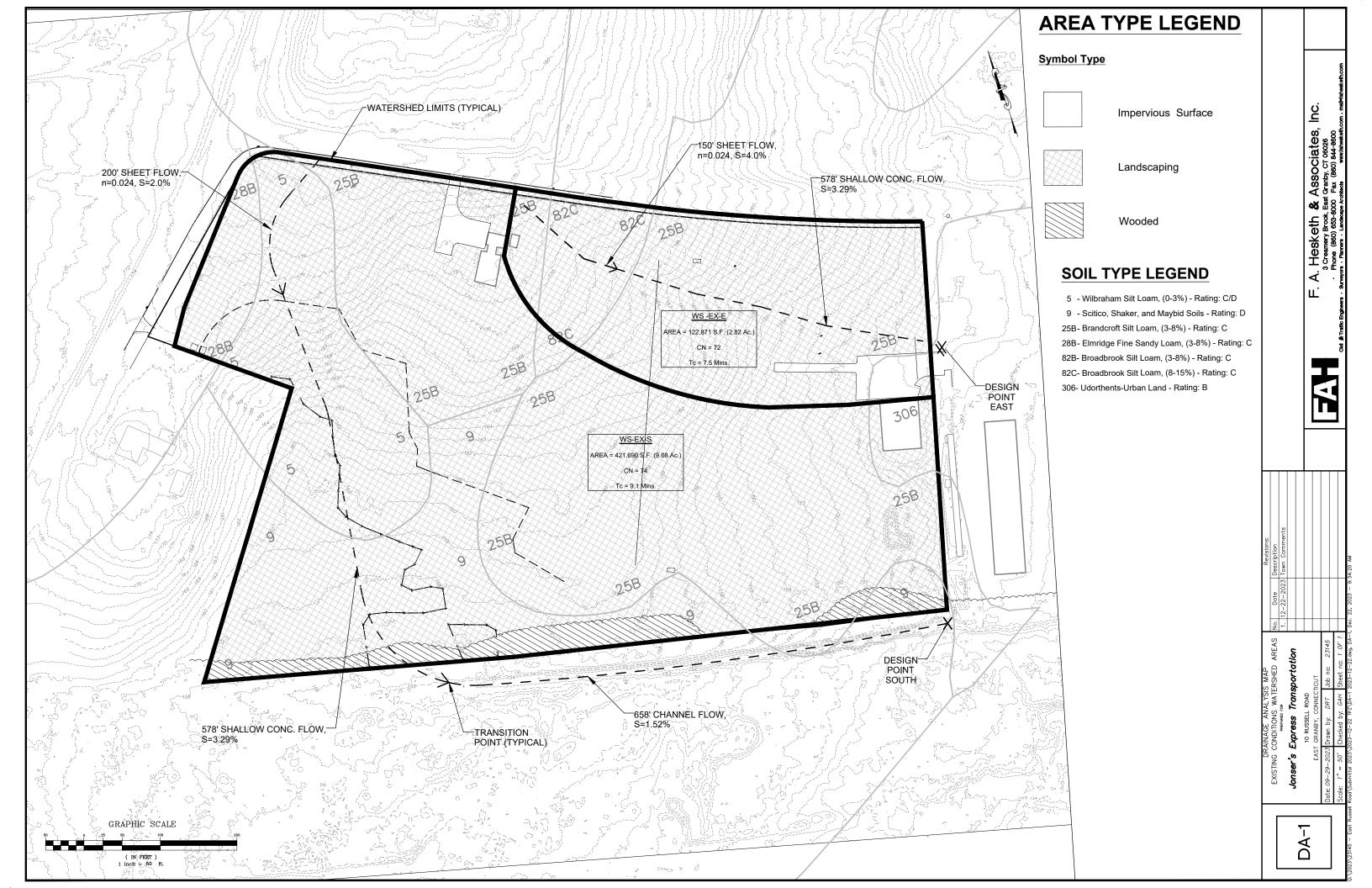


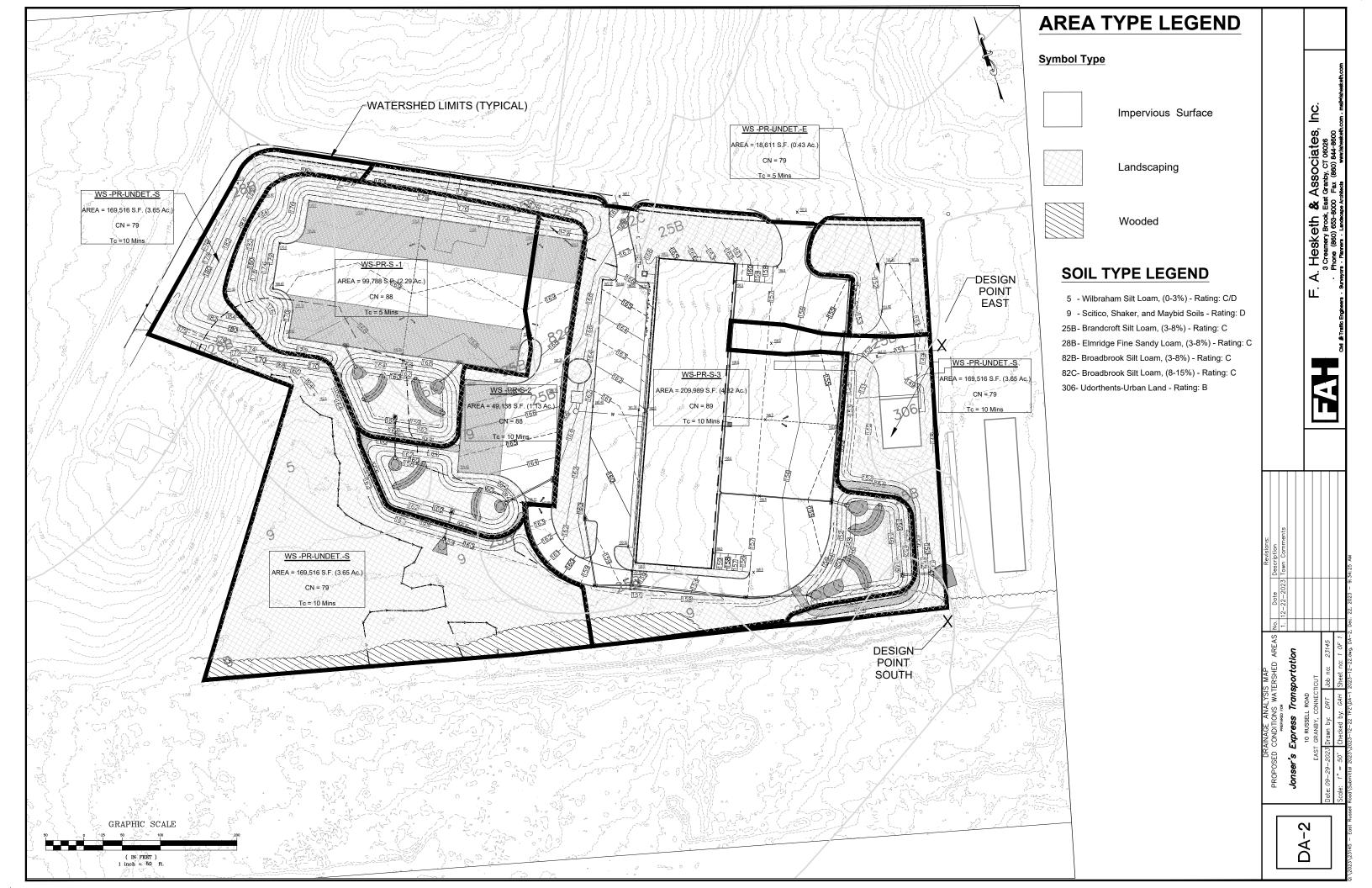


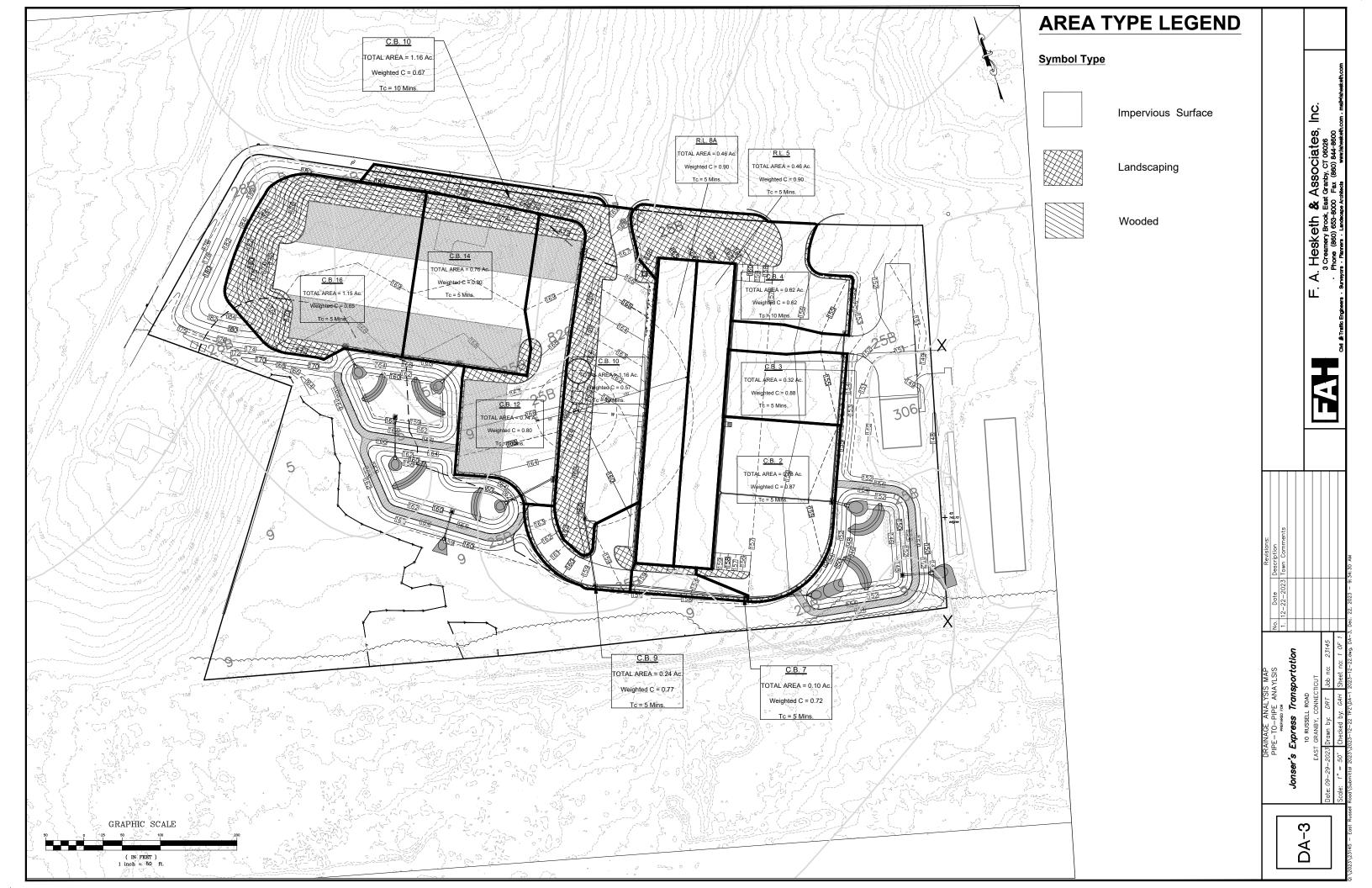


Attachment A

Watershed Area Maps







Attachment B

Soil Type Maps

41° 56′ 44″ N

Web Soil Survey National Cooperative Soil Survey

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41° 56' 30" N

MAP LEGEND

Special Line Features Streams and Canals Interstate Highways Aerial Photography Very Stony Spot Major Roads Local Roads US Routes Stony Spot Spoil Area Wet Spot Other Rails Water Features **Fransportation** Background W 8 ŧ Soil Map Unit Polygons Area of Interest (AOI) Miscellaneous Water Soil Map Unit Points Soil Map Unit Lines Closed Depression Marsh or swamp Perennial Water Mine or Quarry Special Point Features Rock Outcrop Gravelly Spot Sandy Spot Saline Spot Borrow Pit Clay Spot Lava Flow **Gravel Pit** Area of Interest (AOI) Blowout Landfill Soils

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.

contrasting soils that could have been shown at a more detailed Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of

Please rely on the bar scale on each map sheet for map measurements. Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator distance and area. A projection that preserves area, such as the projection, which preserves direction and shape but distorts Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Survey Area Data: Version 22, Sep 12, 2022 State of Connecticut Soil Survey Area:

Soil map units are labeled (as space allows) for map scales

1:50,000 or larger.

Date(s) aerial images were photographed: Jun 14, 2022—Oct 6,

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Severely Eroded Spot

Slide or Slip

Sinkhole

Sodic Spot

Soil Map—State of Connecticut

J.E.T. - EAST RUSSELL ROAD

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI		
5	Wilbraham silt loam, 0 to 3 percent slopes	1.6			
9	Scitico, Shaker, and Maybid soils	1.9	16.1%		
25B	Brancroft silt loam, 3 to 8 percent slopes 5.5		46.0%		
28B	Elmridge fine sandy loam, 3 to 8 percent slopes	0.5	4.1%		
82B	Broadbrook silt loam, 3 to 8 percent slopes	1.7	14.1%		
82C	Broadbrook silt loam, 8 to 15 percent slopes	0.5	4.1%		
306	Udorthents-Urban land complex	0.3	2.5%		
Totals for Area of Interest		11.9	100.0%		

5-Wilbraham silt loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 2wh26

Elevation: 0 to 770 feet

Mean annual precipitation: 36 to 53 inches Mean annual air temperature: 41 to 54 degrees F

Frost-free period: 140 to 220 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Wilbraham and similar soils: 85 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Wilbraham

Setting

Landform: Ground moraines, drumlins, hills, drainageways,

depressions

Landform position (two-dimensional): Footslope, toeslope

Landform position (three-dimensional): Head slope, base slope

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Red coarse-loamy lodgment till derived from basalt

and/or sandstone and shale

Typical profile

Ap - 0 to 8 inches: silt loam
Bw1 - 8 to 19 inches: silt loam
Bw2 - 19 to 25 inches: silt loam
Cd - 25 to 61 inches: gravelly loam

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: 16 to 35 inches to densic material

Drainage class: Poorly drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): very low

to moderately low (0.00 to 0.14 in/hr) Depth to water table: About 0 to 10 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 5.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4w

Hydrologic Soil Group: C/D

Ecological site: F144AY009CT - Wet Till Depressions

Hydric soil rating: Yes

Minor Components

Ludlow

Percent of map unit: 10 percent Landform: Hills, drumlins

Landform position (two-dimensional): Summit, footslope Landform position (three-dimensional): Base slope, crest

Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Menlo

Percent of map unit: 5 percent

Landform: Drainageways, depressions

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Data Source Information

9—Scitico, Shaker, and Maybid soils

Map Unit Setting

National map unit symbol: 9Irq Elevation: 0 to 1,200 feet

Mean annual precipitation: 43 to 50 inches Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Scitico and similar soils: 40 percent Shaker and similar soils: 30 percent Maybid and similar soils: 15 percent Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Scitico

Setting

Landform: Terraces, drainageways, depressions

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Clayey glaciolacustrine deposits

Typical profile

Ap - 0 to 8 inches: silt loam Eg - 8 to 11 inches: silt loam

Bg1 - 11 to 18 inches: silty clay loam Bg2 - 18 to 30 inches: silty clay loam Bg3 - 30 to 38 inches: silty clay Cg1 - 38 to 52 inches: silty clay loam Cg2 - 52 to 65 inches: silty clay

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Poorly drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Very low

to moderately low (0.00 to 0.06 in/hr) Depth to water table: About 0 to 12 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: High (about 11.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4w

Hydrologic Soil Group: D

Ecological site. F145A1004CT - Wet Lake Plain

Hydric soil rating: Yes

Description of Shaker

Setting

Landform: Terraces, drainageways, depressions

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Coarse-loamy eolian deposits over clayey

glaciolacustrine deposits

Typical profile

Oe - 0 to 2 inches: moderately decomposed plant material

Ap - 2 to 6 inches: fine sandy loam Bg - 6 to 20 inches: sandy loam Bw - 20 to 30 inches: sandy loam 2C - 30 to 65 inches: silty clay

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Poorly drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): Very low

to moderately high (0.00 to 0.20 in/hr) Depth to water table: About 0 to 18 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: High (about 9.8 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land canability classification (nonirrigated): 4w

Hydrologic Soil Group: C/D

Ecological site. F144AYU19INH - Wet Lake Plain

Hydric soil rating: Yes

Description of Maybid

Setting

Landform: Terraces, drainageways, depressions

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Clayey glaciolacustrine deposits

Typical profile

A - 0 to 9 inches: silt loam

Bg1 - 9 to 18 inches: silty clay loam Bg2 - 18 to 26 inches: silty clay loam Cg1 - 26 to 36 inches: silty clay loam Cg2 - 36 to 60 inches: silty clay loam



Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Very poorly drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Very low

to moderately high (0.00 to 0.20 in/hr) Depth to water table: About 0 to 6 inches

Frequency of flooding: None Frequency of ponding: Occasional

Available water supply, 0 to 60 inches: High (about 11.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6w

Hydrologic Soil Group: C/D

Ecological site. F145A1005CT - very Wet Inland Lake Plain

Hydric soil rating: Yes

Minor Components

Brancroft

Percent of map unit: 5 percent

Landform: Terraces

Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Elmridge

Percent of map unit: 5 percent

Landform: Terraces

Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Unnamed, sand or gravel substratum

Percent of map unit: 3 percent

Hydric soil rating: No

Unnamed, red parent material

Percent of map unit: 2 percent

Data Source Information

25B—Brancroft silt loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 9ll7 Elevation: 0 to 1,200 feet

Mean annual precipitation: 43 to 52 inches Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Brancroft and similar soils: 80 percent Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Brancroft

Setting

Landform: Terraces

Down-slope shape: Concave Across-slope shape: Linear

Parent material: Fine-silty glaciolacustrine deposits

Typical profile

Ap - 0 to 6 inches: silt loam

Bw1 - 6 to 17 inches: silt loam

Bw2 - 17 to 22 inches: silty clay loam

Bw3 - 22 to 32 inches: silt loam

C1 - 32 to 43 inches: silty clay loam

C2 - 43 to 66 inches: silt loam

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches Drainage class: Moderately well drained

Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Very low

to moderately high (0.00 to 0.57 in/hr) Depth to water table: About 18 to 30 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: High (about 12.0 inches)

Interpretive groups

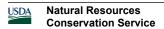
Land capability classification (irrigated): None specified

Land canability classification (nonirrigated): 2e

Hydrologic Soil Group: C

Ecological site. F145AYUUOCT - Semi-Rich Moist Lake Plain

Hydric soil rating: No



Minor Components

Berlin

Percent of map unit: 5 percent

Landform: Terraces
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

Elmridge

Percent of map unit: 5 percent

Landform: Terraces

Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Maybid

Percent of map unit: 3 percent

Landform: Terraces, drainageways, depressions

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Unnamed, till substratum

Percent of map unit: 2 percent

Hydric soil rating: No

Scitico

Percent of map unit: 2 percent

Landform: Terraces, drainageways, depressions

Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

Unnamed, sand or gravel substratum

Percent of map unit: 2 percent

Hydric soil rating: No

Belgrade

Percent of map unit: 1 percent

Landform: Terraces
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

Data Source Information

28B—Elmridge fine sandy loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 9lm1 Elevation: 0 to 1,200 feet

Mean annual precipitation: 43 to 54 inches Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Elmridge and similar soils: 80 percent Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Elmridge

Setting

Landform: Terraces

Down-slope shape: Concave Across-slope shape: Linear

Parent material: Coarse-loamy eolian sands over clayey

glaciolacustrine deposits

Typical profile

Ap - 0 to 6 inches: fine sandy loam Bw1 - 6 to 10 inches: fine sandy loam Bw2 - 10 to 18 inches: fine sandy loam Bw3 - 18 to 25 inches: sandy loam 2C - 25 to 65 inches: silty clay

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches Drainage class: Moderately well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Very low

to moderately low (0.00 to 0.06 in/hr)

Depth to water table: About 18 to 30 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 9.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land canability classification (nonirrigated): 2w

Hydrologic Soil Group: C

Ecological site. F145X1000CT - Semi-Rich Moist Lake Plain

Hydric soil rating: No

Minor Components

Brancroft

Percent of map unit: 5 percent

Landform: Terraces

Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Shaker

Percent of map unit: 3 percent

Landform: Terraces, drainageways, depressions

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Unnamed, red parent material

Percent of map unit: 2 percent

Hydric soil rating: No

Scitico

Percent of map unit: 2 percent

Landform: Terraces, drainageways, depressions

Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

Sudbury

Percent of map unit: 2 percent Landform: Terraces, outwash plains

Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Belgrade

Percent of map unit: 2 percent

Landform: Terraces
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

Ninigret

Percent of map unit: 2 percent Landform: Terraces, outwash plains

Down-slope shape: Linear Across-slope shape: Concave

Hydric soil rating: No

Berlin

Percent of map unit: 1 percent

Landform: Terraces
Down-slope shape: Linear
Across-slope shape: Linear

Hydric soil rating: No

Maybid

Percent of map unit: 1 percent Landform: Terraces, drainageways, depressions Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

Data Source Information

82B—Broadbrook silt loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 9lr3 Elevation: 0 to 1,200 feet

Mean annual precipitation: 40 to 54 inches
Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Broadbrook and similar soils: 80 percent

Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Broadbrook

Setting

Landform: Till plains, hills, drumlins

Down-slope shape: Linear Across-slope shape: Concave

Parent material: Eolian deposits over coarse-loamy lodgment till derived from gneiss and/or schist and/or sandstone and/or

basalt

Typical profile

Ap - 0 to 8 inches: silt loam Bw1 - 8 to 14 inches: silt loam Bw2 - 14 to 25 inches: silt loam

2Cd - 25 to 65 inches: gravelly fine sandy loam

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: 20 to 40 inches to densic material

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Very low

to moderately high (0.00 to 0.20 in/hr) Depth to water table: About 18 to 30 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 4.8 inches)

Interpretive groups

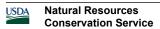
Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: C

Ecological site: F145XY012C1 - Well Drained Dense Till Uplands

Hydric soil rating: No



Minor Components

Rainbow

Percent of map unit: 5 percent Landform: Hills, drumlins Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: No

Wethersfield

Percent of map unit: 5 percent Landform: Hills, drumlins Down-slope shape: Linear Across-slope shape: Convex Hydric soil rating: No

Holyoke

Percent of map unit: 3 percent Landform: Ridges, hills Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

Wilbraham

Percent of map unit: 3 percent Landform: Drainageways, depressions Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

Menlo

Percent of map unit: 2 percent Landform: Drainageways, depressions Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

Narragansett

Percent of map unit: 2 percent Landform: Till plains, hills Down-slope shape: Linear Across-slope shape: Convex Hydric soil rating: No

Data Source Information

82C—Broadbrook silt loam, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 9lr4 Elevation: 0 to 1,200 feet

Mean annual precipitation: 40 to 54 inches
Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Broadbrook and similar soils: 80 percent

Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Broadbrook

Setting

Landform: Till plains, hills, drumlins

Down-slope shape: Linear Across-slope shape: Concave

Parent material: Eolian deposits over coarse-loamy lodgment till derived from gneiss and/or schist and/or sandstone and/or

basalt

Typical profile

Ap - 0 to 8 inches: silt loam
Bw1 - 8 to 14 inches: silt loam
Bw2 - 14 to 25 inches: silt loam

2Cd - 25 to 65 inches: gravelly fine sandy loam

Properties and qualities

Slope: 8 to 15 percent

Depth to restrictive feature: 20 to 40 inches to densic material

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Very low

to moderately high (0.00 to 0.20 in/hr) Depth to water table: About 18 to 30 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 4.8 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: C

Ecological site: F145XY012C1 - Well Drained Dense Till Uplands

Hydric soil rating: No

Minor Components

Rainbow

Percent of map unit: 5 percent Landform: Hills, drumlins Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: No

Wethersfield

Percent of map unit: 5 percent Landform: Hills, drumlins Down-slope shape: Linear Across-slope shape: Convex Hydric soil rating: No

Wilbraham

Percent of map unit: 3 percent Landform: Drainageways, depressions Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

Holyoke

Percent of map unit: 3 percent Landform: Ridges, hills Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

Narragansett

Percent of map unit: 2 percent Landform: Till plains, hills Down-slope shape: Linear Across-slope shape: Convex Hydric soil rating: No

Menlo

Percent of map unit: 2 percent Landform: Drainageways, depressions Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

Data Source Information

306—Udorthents-Urban land complex

Map Unit Setting

National map unit symbol: 9lmg Elevation: 0 to 2,000 feet

Mean annual precipitation: 43 to 56 inches Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 120 to 185 days

Farmland classification: Not prime farmland

Map Unit Composition

Udorthents and similar soils: 50 percent

Urban land: 35 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Udorthents

Setting

Down-slope shape: Convex Across-slope shape: Linear Parent material: Drift

Typical profile

A - 0 to 5 inches: loam

C1 - 5 to 21 inches: gravelly loam

C2 - 21 to 80 inches: very gravelly sandy loam

Properties and qualities

Slope: 0 to 25 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Very low

to high (0.00 to 1.98 in/hr)

Depth to water table: About 54 to 72 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 6.8

inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: B

Hydric soil rating: No

Description of Urban Land

Typical profile

H - 0 to 6 inches: material

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 8

Hydrologic Soil Group: D Hydric soil rating: Unranked

Minor Components

Unnamed, undisturbed soils

Percent of map unit: 8 percent Hydric soil rating: No

Udorthents, wet substratum

Percent of map unit: 5 percent Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

Rock outcrop

Percent of map unit: 2 percent Hydric soil rating: No

Data Source Information

Attachment C

NOAA Rainfall Data



NOAA Atlas 14, Volume 10, Version 3 Location name: East Granby, Connecticut, USA* Latitude: 41.9443°, Longitude: -72.7107° Elevation: 172 ft**



* source: ESRI Maps ** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

Duration F	Average recurrence interval (years)									
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	4.18 (3.19-5.44)	5.00 (3.83-6.52)	6.36 (4.85-8.32)	7.48 (5.66-9.83)	9.01 (6.64-12.4)	10.2 (7.36-14.3)	11.4 (8.03-16.6)	12.7 (8.54-19.0)	14.7 (9.50-22.7)	16.3 (10.3-25.7)
10-min	2.96 (2.26-3.85)	3.55 (2.71-4.62)	4.50 (3.43-5.89)	5.29 (4.01-6.97)	6.38 (4.70-8.78)	7.21 (5.21-10.1)	8.07 (5.68-11.8)	9.04 (6.05-13.5)	10.4 (6.73-16.1)	11.5 (7.30-18.2)
15-min	2.32 (1.78-3.02)	2.78 (2.12-3.62)	3.53 (2.69-4.62)	4.15 (3.15-5.46)	5.01 (3.69-6.89)	5.65 (4.09-7.95)	6.33 (4.46-9.23)	7.08 (4.74-10.6)	8.16 (5.28-12.6)	9.05 (5.73-14.3
30-min	1.56 (1.19-2.03)	1.88 (1.44-2.45)	2.40 (1.83-3.14)	2.83 (2.15-3.72)	3.43 (2.52-4.71)	3.88 (2.80-5.44)	4.34 (3.06-6.33)	4.87 (3.26-7.27)	5.61 (3.63-8.69)	6.22 (3.93-9.82)
60-min	0.978 (0.748-1.27)	1.18 (0.904-1.54)	1.52 (1.16-1.98)	1.79 (1.36-2.36)	2.17 (1.60-2.99)	2.46 (1.78-3.46)	2.76 (1.94-4.03)	3.10 (2.07-4.62)	3.57 (2.31-5.52)	3.96 (2.50-6.25
2-hr	0.631 (0.486-0.816)	0.759 (0.584-0.982)	0.967 (0.742-1.26)	1.14 (0.870-1.49)	1.38 (1.02-1.89)	1.56 (1.13-2.18)	1.75 (1.24-2.55)	1.97 (1.32-2.92)	2.29 (1.49-3.53)	2.56 (1.63-4.03)
3-hr	0.484 (0.374-0.623)	0.582 (0.449-0.750)	0.742 (0.571-0.961)	0.876 (0.670-1.14)	1.06 (0.789-1.45)	1.20 (0.875-1.67)	1.34 (0.959-1.96)	1.52 (1.02-2.24)	1.78 (1.16-2.73)	2.00 (1.27-3.13)
6-hr	0.304 (0.236-0.389)	0.369 (0.286-0.472)	0.474 (0.367-0.610)	0.562 (0.433-0.727)	0.682 (0.512-0.929)	0.771 (0.569-1.08)	0.868 (0.627-1.27)	0.988 (0.668-1.45)	1.17 (0.764-1.79)	1.33 (0.849-2.07
12-hr	0.184 (0.144-0.235)	0.227 (0.178-0.289)	0.297 (0.231-0.379)	0.355 (0.275-0.456)	0.434 (0.328-0.589)	0.493 (0.366-0.685)	0.557 (0.405-0.811)	0.638 (0.433-0.934)	0.765 (0.500-1.16)	0.875 (0.560-1.35
24-hr	0.107 (0.084-0.135)	0.134 (0.106-0.170)	0.179 (0.140-0.227)	0.215 (0.168-0.275)	0.266 (0.202-0.359)	0.303 (0.227-0.420)	0.344 (0.253-0.501)	0.397 (0.270-0.579)	0.483 (0.316-0.728)	0.558 (0.358-0.85
2-day	0.059 (0.047-0.075)	0.076 (0.060-0.095)	0.102 (0.081-0.129)	0.124 (0.097-0.157)	0.154 (0.118-0.208)	0.176 (0.133-0.244)	0.201 (0.149-0.293)	0.234 (0.160-0.339)	0.289 (0.190-0.433)	0.338 (0.217-0.51
3-day	0.043 (0.034-0.054)	0.055 (0.044-0.069)	0.074 (0.059-0.094)	0.091 (0.071-0.114)	0.113 (0.087-0.152)	0.129 (0.098-0.178)	0.147 (0.110-0.214)	0.172 (0.117-0.248)	0.213 (0.140-0.318)	0.249 (0.161-0.38
4-day	0.035 (0.028-0.043)	0.044 (0.035-0.055)	0.060 (0.048-0.075)	0.073 (0.058-0.092)	0.091 (0.070-0.122)	0.104 (0.079-0.143)	0.118 (0.088-0.172)	0.138 (0.095-0.199)	0.171 (0.113-0.256)	0.201 (0.130-0.30
7-day	0.024 (0.019-0.030)	0.030 (0.024-0.037)	0.040 (0.032-0.050)	0.049 (0.039-0.061)	0.060 (0.047-0.080)	0.069 (0.052-0.094)	0.078 (0.059-0.113)	0.091 (0.063-0.131)	0.112 (0.074-0.167)	0.131 (0.085-0.19
10-day	0.019 (0.015-0.024)	0.024 (0.019-0.030)	0.032 (0.025-0.039)	0.038 (0.030-0.047)	0.047 (0.036-0.062)	0.053 (0.040-0.072)	0.060 (0.045-0.086)	0.069 (0.048-0.099)	0.085 (0.056-0.126)	0.098 (0.064-0.14
20-day	0.014 (0.011-0.017)	0.016 (0.013-0.020)	0.020 (0.016-0.025)	0.024 (0.019-0.029)	0.028 (0.022-0.037)	0.032 (0.024-0.042)	0.035 (0.026-0.050)	0.040 (0.028-0.057)	0.047 (0.031-0.069)	0.054 (0.035-0.08
30-day	0.012 (0.009-0.014)	0.013 (0.011-0.016)	0.016 (0.013-0.020)	0.018 (0.015-0.023)	0.021 (0.016-0.028)	0.024 (0.018-0.031)	0.026 (0.019-0.036)	0.029 (0.020-0.041)	0.033 (0.022-0.049)	0.037 (0.024-0.05
45-day	0.010 (0.008-0.012)	0.011 (0.009-0.013)	0.013 (0.010-0.016)	0.014 (0.011-0.018)	0.016 (0.013-0.021)	0.018 (0.014-0.024)	0.020 (0.014-0.027)	0.021 (0.015-0.030)	0.024 (0.016-0.035)	0.026 (0.017-0.03
60-day	0.008	0.009	0.011 (0.009-0.013)	0.012	0.014	0.015	0.016	0.017	0.019	0.020

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

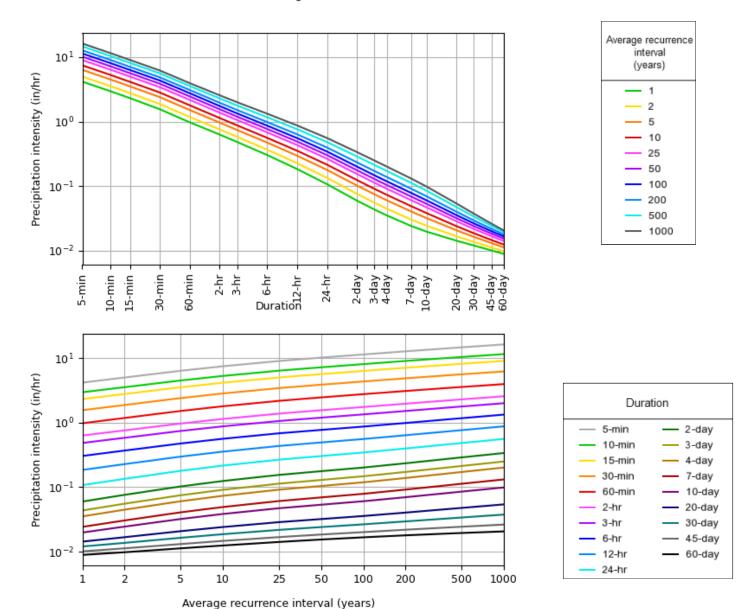
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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PF graphical

PDS-based intensity-duration-frequency (IDF) curves Latitude: 41.9443°, Longitude: -72.7107°



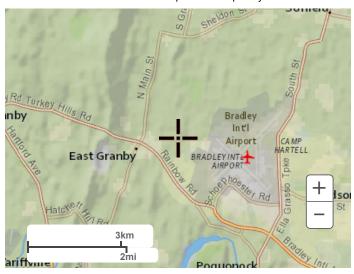
NOAA Atlas 14, Volume 10, Version 3

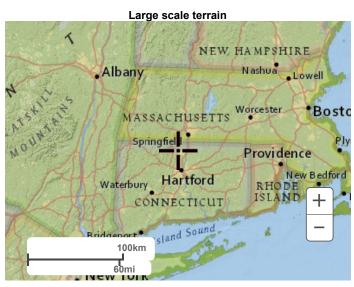
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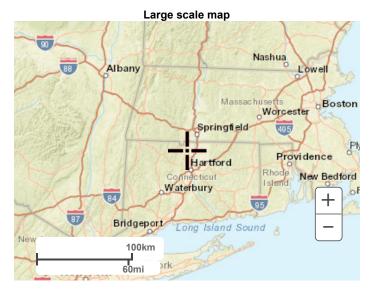
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Maps & aerials

Small scale terrain







Large scale aerial



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<u>Disclaimer</u>



NOAA Atlas 14, Volume 10, Version 3 Location name: East Granby, Connecticut, USA* Latitude: 41.9443°, Longitude: -72.7107° Elevation: 172 ft**



* source: ESRI Maps ** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

PDS-k	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹									
Duration				Average	recurrence	interval (y	ears)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.348 (0.266-0.453)	0.417 (0.319-0.543)	0.530 (0.404-0.693)	0.623 (0.472-0.819)	0.751 (0.553-1.03)	0.848 (0.613-1.19)	0.949 (0.669-1.38)	1.06 (0.712-1.59)	1.22 (0.792-1.90)	1.36 (0.859-2.14)
10-min	0.493 (0.377-0.642)	0.591 (0.452-0.770)	0.750 (0.572-0.982)	0.882 (0.669-1.16)	1.06 (0.784-1.46)	1.20 (0.868-1.69)	1.34 (0.947-1.96)	1.51 (1.01-2.25)	1.74 (1.12-2.68)	1.92 (1.22-3.04)
15-min	0.580 (0.444-0.755)	0.695 (0.531-0.906)	0.883 (0.673-1.16)	1.04 (0.787-1.36)	1.25 (0.922-1.72)	1.41 (1.02-1.99)	1.58 (1.11-2.31)	1.77 (1.19-2.64)	2.04 (1.32-3.16)	2.26 (1.43-3.57)
30-min	0.779 (0.596-1.01)	0.939 (0.718-1.22)	1.20 (0.915-1.57)	1.42 (1.07-1.86)	1.71 (1.26-2.36)	1.94 (1.40-2.72)	2.17 (1.53-3.17)	2.43 (1.63-3.63)	2.80 (1.82-4.34)	3.11 (1.97-4.91)
60-min	0.978 (0.748-1.27)	1.18 (0.904-1.54)	1.52 (1.16-1.98)	1.79 (1.36-2.36)	2.17 (1.60-2.99)	2.46 (1.78-3.46)	2.76 (1.94-4.03)	3.10 (2.07-4.62)	3.57 (2.31-5.52)	3.96 (2.50-6.25)
2-hr	1.26 (0.972-1.63)	1.52 (1.17-1.96)	1.94 (1.48-2.51)	2.28 (1.74-2.98)	2.76 (2.05-3.78)	3.12 (2.27-4.36)	3.49 (2.48-5.10)	3.94 (2.64-5.84)	4.58 (2.97-7.06)	5.13 (3.26-8.06)
3-hr	1.45 (1.12-1.87)	1.75 (1.35-2.26)	2.23 (1.72-2.89)	2.63 (2.01-3.42)	3.18 (2.37-4.35)	3.59 (2.63-5.02)	4.03 (2.88-5.88)	4.55 (3.07-6.74)	5.34 (3.47-8.20)	6.01 (3.82-9.41)
6-hr	1.82 (1.42-2.33)	2.21 (1.72-2.83)	2.84 (2.20-3.66)	3.37 (2.60-4.36)	4.09 (3.07-5.56)	4.62 (3.41-6.45)	5.20 (3.76-7.58)	5.92 (4.00-8.71)	7.02 (4.58-10.7)	7.97 (5.09-12.4)
12-hr	2.23 (1.74-2.83)	2.74 (2.14-3.49)	3.58 (2.79-4.58)	4.28 (3.32-5.50)	5.24 (3.96-7.10)	5.94 (4.42-8.26)	6.72 (4.89-9.77)	7.69 (5.22-11.3)	9.22 (6.03-14.0)	10.6 (6.75-16.3)
24-hr	2.58 (2.04-3.26)	3.23 (2.55-4.09)	4.30 (3.37-5.45)	5.18 (4.04-6.61)	6.40 (4.87-8.64)	7.28 (5.46-10.1)	8.27 (6.08-12.0)	9.55 (6.50-13.9)	11.6 (7.60-17.5)	13.4 (8.60-20.6)
2-day	2.87 (2.28-3.60)	3.65 (2.89-4.58)	4.92 (3.89-6.21)	5.98 (4.70-7.58)	7.43 (5.70-10.0)	8.49 (6.41-11.8)	9.68 (7.19-14.1)	11.3 (7.69-16.3)	13.9 (9.13-20.8)	16.2 (10.5-24.8)
3-day	3.13 (2.50-3.91)	3.99 (3.18-4.99)	5.39 (4.28-6.77)	6.56 (5.17-8.28)	8.16 (6.28-10.9)	9.31 (7.07-12.9)	10.6 (7.93-15.5)	12.4 (8.48-17.9)	15.3 (10.1-22.9)	18.0 (11.6-27.4)
4-day	3.38 (2.70-4.21)	4.30 (3.43-5.36)	5.81 (4.62-7.27)	7.05 (5.58-8.88)	8.77 (6.77-11.7)	10.0 (7.62-13.8)	11.4 (8.54-16.6)	13.3 (9.13-19.2)	16.5 (10.9-24.6)	19.3 (12.5-29.4)
7-day	4.07 (3.27-5.05)	5.12 (4.11-6.36)	6.84 (5.47-8.52)	8.27 (6.57-10.4)	10.2 (7.93-13.6)	11.7 (8.89-15.9)	13.3 (9.94-19.1)	15.4 (10.6-22.1)	19.0 (12.5-28.1)	22.2 (14.3-33.5)
10-day	4.76 (3.83-5.87)	5.88 (4.73-7.26)	7.70 (6.18-9.55)	9.22 (7.35-11.5)	11.3 (8.79-15.0)	12.8 (9.80-17.4)	14.5 (10.9-20.8)	16.8 (11.6-23.9)	20.5 (13.6-30.2)	23.8 (15.4-35.8)
20-day	6.88 (5.58-8.43)	8.06 (6.53-9.88)	9.98 (8.06-12.3)	11.6 (9.29-14.3)	13.8 (10.7-18.0)	15.4 (11.8-20.6)	17.2 (12.8-24.0)	19.4 (13.5-27.4)	22.9 (15.3-33.6)	26.0 (16.9-38.9)
30-day	8.68 (7.07-10.6)	9.88 (8.04-12.1)	11.8 (9.59-14.5)	13.5 (10.8-16.6)	15.7 (12.2-20.3)	17.3 (13.2-23.0)	19.1 (14.2-26.4)	21.2 (14.8-29.9)	24.4 (16.3-35.6)	27.1 (17.6-40.4)
45-day	10.9 (8.95-13.3)	12.2 (9.94-14.8)	14.2 (11.5-17.3)	15.9 (12.8-19.5)	18.2 (14.2-23.2)	19.9 (15.2-26.0)	21.7 (16.0-29.4)	23.7 (16.6-33.1)	26.3 (17.7-38.2)	28.4 (18.6-42.3)
60-day	12.8 (10.5-15.6)	14.1 (11.6-17.1)	16.2 (13.2-19.8)	18.0 (14.6-22.0)	20.4 (15.9-25.9)	22.2 (16.9-28.8)	24.1 (17.6-32.2)	25.8 (18.1-36.0)	28.1 (18.9-40.6)	29.7 (19.4-44.0)

Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

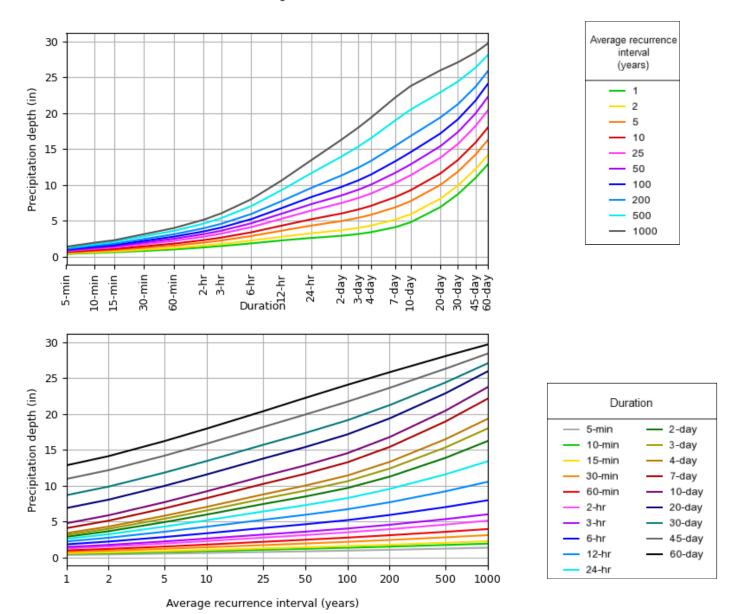
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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PF graphical

PDS-based depth-duration-frequency (DDF) curves Latitude: 41.9443°, Longitude: -72.7107°



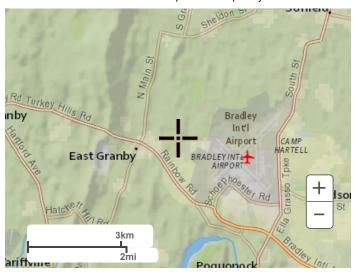
NOAA Atlas 14, Volume 10, Version 3

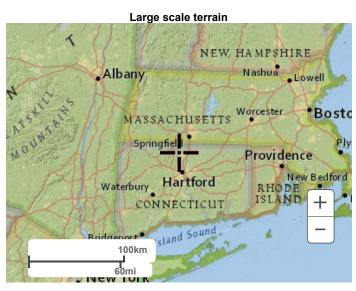
Created (GMT): Wed Sep 20 12:29:38 2023

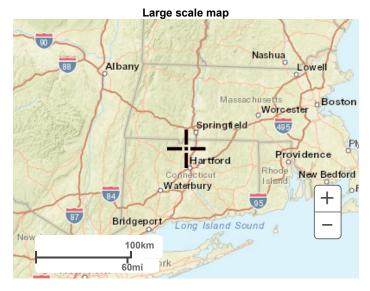
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Maps & aerials

Small scale terrain







Large scale aerial



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US Department of Commerce
National Oceanic and Atmospheric Administration
National Weather Service
National Water Center
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

<u>Disclaimer</u>

Attachment D

Hydrologic Analysis

Project:	J.E.T. 10 Russell Road		By:	DRT		Date:	12/22/2023	
Location:	East Granby, CT.		Checked:	GAH		Date:		
Check one	Pre	esent	X	Developed		WS-EX-E		
1. Runoff cu	rve number							
Soil name and hydrologic group (appendix A)	Cover des (cover type, treatment, and h impervious; unconnected/com	ydrologic co	ondition;percent			CN	Area □ acres ☒ ft² □ %	Area X acres mi ² %
	IMPERVIOUS (GOOD)					98	6,633	0.15
С	MEADOW : NON-GRAZED (GOOD)					71	116,237	2.67
Use only one CN sou	rce per line					Totals	122,871	2.82
	tota	ıl produc		204.38				
CN (weighted)		ıl area	=	2.82	=	72.46	Use CN	72

Project:	J.E.T. 10 Russell Road		By:	DRT		Date:	12/22/2023	
Location:	East Granby, CT.		Checked:	GAH		Date:		
Check one	P	resent	X	Developed		WS-EX-S		
1. Runoff cu	rve number							
Soil name and hydrologic group (appendix A)	Cover de (cover type, treatment, and impervious; unconnected/ce		ondition;percent			CN	Area □ acres ☒ ft² □ %	Area
	IMPERVIOUS (GOOD)					98	8,295	0.19
	MEADOW : NON-GRAZED (GOOD)					58	5,495	0.13
С	MEADOW : NON-GRAZED (GOOD)					71	234,754	5.39
D	MEADOW : NON-GRAZED (GOOD)					78	151,066	3.47
С	WOODS (GOOD)					70	1,607	0.04
D	WOODS (GOOD)					77	20,473	0.47
						Totals	421 690	0.68
Use only one CN sou	•					Totals	421,690	9.68
CN (weighted)	· = -	tal produc	<u>t</u> =	717.89 ————————————————————————————————————	=	74.16	Use CN	74

Project:	J.E.T. 10 Russell Road	By:	DRT	Date:	12/22/2023		
Location:	East Granby, CT.	Checked:	GAH	Date:			
Check one	Pres	sent	Developed X	WS-PR-UN	NDET-E		
1. Runoff cu	rve number						
Soil name and	Cover des	cription		CN	Area	Area	Product of
hydrologic group	(cover type, treatment, and hy	drologic condition;percent			□ acres x ft²	x acres □ mi²	CN x area
(appendix A)	impervious; unconnected/conne				□ %	□ %	
	IMPERVIOUS (GOOD)			98	3,837	0.09	8.63
С	MEADOW : NON-GRAZED (GOOD)			71	14,774	0.34	24.08
					1		
Use only one CN sou	rce per line			Totals	18,611	0.43	32.71
CDL (product	32.71	74.55			
CN (weighted)		area	0.43	76.57		Use CN	77

Project:	J.E.T. 10 Russell Road	Ву:	DRT		Date:	12/22/2023		
Location:	East Granby, CT.	Checked:	GAH		Date:			
Check one	P	resent	Developed 2	ζ	WS-PR-UN	NDET-S		
1. Runoff cu	rve number							
Soil name and hydrologic group		escription			CN	Area □ acres ☒ ft²	Area	Product of CN x area
(appendix A)		nnected impervious area ratio)				□ %	□ %	
D	IMPERVIOUS (GOOD)				98	14,088	0.32	31.70
B C	MEADOW : NON-GRAZED (GOOD) MEADOW : NON-GRAZED (GOOD)				58 71	7,250 41,490	0.17 0.95	9.65 67.63
D	MEADOW: NON-GRAZED (GOOD)				78	93,845	2.15	168.04
D	WOODS (GOOD)				77	12,841	0.29	22.70
Use only one CN sou	rce per line				Totals	169,516	3.89	299.72
CN (weighted)		tal product =	299.72	=	77.02	Use CN	77	
. (8		tal area	3.89		· · ·			

Project:	J.E.T. 10 Russell Road	By:	DRT		Date:	12/22/2023		
Location:	East Granby, CT.	Checked:	GAH		Date:			
Check one	1	Present	Developed X		WS-PR-S-1			
1. Runoff cu	rve number							
Soil name and hydrologic group (appendix A)	(cover type, treatment, an	d hydrologic condition;percent connected impervious area ratio)			CN	Area □ acres ⋉ ft² □ %	Area x acres mi ² %	Product of CN x area
В	IMPERVIOUS (GOOD)				98	56,874	1.31	127.95
С	MEADOW : NON-GRAZED (GOOD)				71	21,027	0.48	34.27
D	MEADOW : NON-GRAZED (GOOD)				78	21,887	0.50	39.19
Use only one CN sou	rce per line				Totals	99,788	2.29	201.42
	to	otal product	201.42					- '-
CN (weighted)		otal area	2.29	=	87.92	Use CN	88	

Project:	J.E.T. 10 Russell Road	By:	DRT		Date:	12/22/2023	
Location:	East Granby, CT.	Checked:	GAH		Date:		
Check one	Present		Developed	X	WS-PR-S-2		
1. Runoff cu	ırve number						
Soil name and hydrologic group (appendix A)	Cover descripti (cover type, treatment, and hydrologic impervious; unconnected/connected im	condition;percent)		CN	Area □ acres □ ### ################################	Area X acres mi ² %
	IMPERVIOUS (GOOD)				98	26,880	0.62
С	MEADOW: NON-GRAZED (GOOD)				71	8,723	0.20
D	MEADOW : NON-GRAZED (GOOD)				78	13,535	0.31
Use only one CN so	I urce per line				Totals	49,138	1.13
CN (weighted	total prod	uct =	98.93	=	87.70	Use CN	88

total area

1.13

Project:	J.E.T. 10 Russell Road	By:	DRT	Date:	12/22/2023		
Location:	East Granby, CT.	Checked:	GAH	Date:			
Check one		Present	Developed X	WS-PR-S-	3		
1. Runoff cu	irve number						
Soil name and	(Cover description		CN	Area	Area	Product of
hydrologic group (appendix A)		ment, and hydrologic condition;percent))		□ acres x ft² □ %	x acres mi² %	CN x area
	IMPERVIOUS (GOOD)			98	133,525	3.07	300.40
С	MEADOW : NON-GRAZED (GOOD)			71	65,374	1.50	106.56
D	MEADOW : NON-GRAZED (GOOD)			78	6,609	0.15	11.83
D	WOODS (GOOD)			77	4,480	0.10	7.92
Use only one CN so	urce per line			Totals	209,988	4.82	426.71
CN (weighted) =	total product =	426.71 =	88.52	Use CN	89	

4.82

total area

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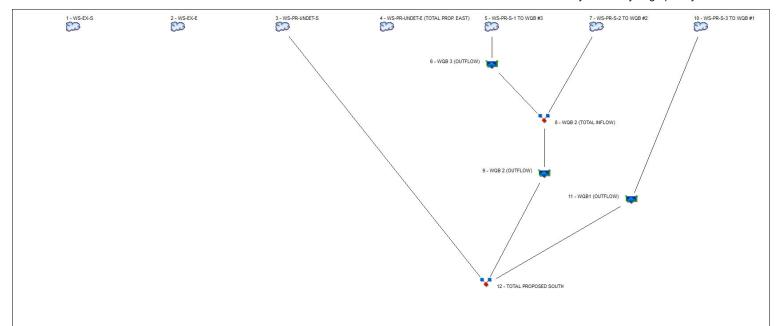
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Watershed Model Schematic

Hydraflow Hydrographs by Intelisolve v9.1



Legend

<u>Hyd.</u>	<u>Origin</u>	<u>Description</u>
1	SCS Runoff	WS-EX-S
2	SCS Runoff	WS-EX-E
3	SCS Runoff	WS-PR-UNDET-S
4	SCS Runoff	WS-PR-UNDET-E (TOTAL PROP. EAST)
5	SCS Runoff	WS-PR-S-1 TO WQB #3
6	Reservoir	WQB 3 (OUTFLOW)
7	SCS Runoff	WS-PR-S-2 TO WQB #2
8	Combine	WQB 2 (TOTAL INFLOW)
9	Reservoir	WQB 2 (OUTFLOW)
10	SCS Runoff	WS-PR-S-3 TO WQB #1
11	Reservoir	WQB1 (OUTFLOW)
12	Combine	TOTAL PROPOSED SOUTH

Project: Macro Model 2023-12-22.gpw

Friday, Dec 22, 2023

Hydrograph Return Period Recap

Hydraflow Hydrographs by Intelisolve v9.1

No. type (origin) Hyd(s) 1-Yr 2-Yr 3-Yr 5-Yr 10-Yr 25-Yr 50-Yr 100-Yr description 1 SCS Runoff	
2 SCS Runoff 3.128 5.587 7.698 10.94 13.56 15.96 WS-EX-E 3 SCS Runoff 4.955 8.420 11.34 15.77 19.31 22.54 WS-PR-UNDET-S 4 SCS Runoff	
3 SCS Runoff 4.955 8.420 11.34 15.77 19.31 22.54 WS-PR-UNDET-S 4 SCS Runoff 0.630 1.068 1.437 1.996 2.442 2.849 WS-PR-UNDET-E (TOTAL INFLOW) 5 SCS Runoff 5.598 8.217 10.31 13.38 15.78 17.94 WS-PR-S-1 TO WQB #3 6 Reservoir 5 0.568 0.948 1.166 1.410 1.567 1.682 WQB 3 (OUTFLOW) 7 SCS Runoff 2.762 4.054 5.088 6.603 7.784 8.852 WS-PR-S-2 TO WQB #2 8 Combine 6, 7 2.777 4.598 5.901 7.653 8.978 10.15 WQB 2 (TOTAL INFLOW) 9 Reservoir 8 0.502 0.851 1.047 1.290 1.441 1.567 WQB 2 (OUTFLOW) 10 SCS Runoff 10.69	
4 SCS Runoff 0.630 1.068 1.437 1.996 2.442 2.849 WS-PR-UNDET-E (TOTAL SCS Runoff 5 SCS Runoff 5.598 8.217 10.31 13.38 15.78 17.94 WS-PR-S-1 TO WQB #3 6 Reservoir 5 0.568 0.948 1.166 1.410 1.567 1.682 WQB 3 (OUTFLOW) 7 SCS Runoff 2.762 4.054 5.088 6.603 7.784 8.852 WS-PR-S-2 TO WQB #2 8 Combine 6, 7 2.777 4.598 5.901 7.653 8.978 10.15 WQB 2 (TOTAL INFLOW) 9 Reservoir 8 0.502 0.851 1.047 1.290 1.441 1.567 WQB 2 (OUTFLOW) 10 SCS Runoff 10.69 15.52 19.35 24.98 29.37 33.33 WS-PR-S-3 TO WQB #1 11 Reservoir 10 3.68	
5 SCS Runoff 5.598 8.217 10.31 13.38 15.78 17.94 WS-PR-S-1 TO WQB #3 6 Reservoir 5 0.568 0.948 1.166 1.410 1.567 1.682 WQB 3 (OUTFLOW) 7 SCS Runoff 2.762 4.054 5.088 6.603 7.784 8.852 WS-PR-S-2 TO WQB #2 8 Combine 6, 7 2.777 4.598 5.901 7.653 8.978 10.15 WQB 2 (TOTAL INFLOW) 9 Reservoir 8 0.502 0.851 1.047 1.290 1.441 1.567 WQB 2 (OUTFLOW) 10 SCS Runoff 10.69 15.52 19.35 24.98 29.37 33.33 WS-PR-S-3 TO WQB #1 11 Reservoir 10 3.681 5.713 6.885 8.290 9.190 9.891	
66 Reservoir 5 0.568 0.948 1.166 1.410 1.567 1.682 WQB 3 (OUTFLOW) 7 SCS Runoff 2.762 4.054 5.088 6.603 7.784 8.852 WS-PR-S-2 TO WQB #2 8 Combine 6, 7 2.777 4.598 5.901 7.653 8.978 10.15 WQB 2 (TOTAL INFLOW) 9 Reservoir 8 0.502 0.851 1.047 1.290 1.441 1.567 WQB 2 (OUTFLOW) 10 SCS Runoff 10.69 15.52 19.35 24.98 29.37 33.33 WS-PR-S-3 TO WQB #1 11 Reservoir 10 3.681 5.713 6.885 8.290 9.190 9.891 WQB1 (OUTFLOW)	L PROP. E.
7 SCS Runoff 2.762 4.054 5.088 6.603 7.784 8.852 WS-PR-S-2 TO WQB #2 8 Combine 6, 7 2.777 4.598 5.901 7.653 8.978 10.15 WQB 2 (TOTAL INFLOW 9 Reservoir 8 0.502 0.851 1.047 1.290 1.441 1.567 WQB 2 (OUTFLOW) 10 SCS Runoff 10.69 15.52 19.35 24.98 29.37 33.33 WS-PR-S-3 TO WQB #1 11 Reservoir 10 3.681 5.713 6.885 8.290 9.190 9.891 WQB1 (OUTFLOW)	
8 Combine 6, 7 2.777 4.598 5.901 7.653 8.978 10.15 WQB 2 (TOTAL INFLOW 9 Reservoir 8 0.502 0.851 1.047 1.290 1.441 1.567 WQB 2 (OUTFLOW) 10 SCS Runoff 10.69 15.52 19.35 24.98 29.37 33.33 WS-PR-S-3 TO WQB #1 11 Reservoir 10 3.681 5.713 6.885 8.290 9.190 9.891 WQB1 (OUTFLOW)	
9 Reservoir 8 0.502 0.851 1.047 1.290 1.441 1.567 WQB 2 (OUTFLOW) 10 SCS Runoff 10.69 15.52 19.35 24.98 29.37 33.33 WS-PR-S-3 TO WQB #1 11 Reservoir 10 3.681 5.713 6.885 8.290 9.190 9.891 WQB1 (OUTFLOW)	
10 SCS Runoff 10.69 15.52 19.35 24.98 29.37 33.33 WS-PR-S-3 TO WQB #1 11 Reservoir 10 3.681 5.713 6.885 8.290 9.190 9.891 WQB1 (OUTFLOW))
11 Reservoir 10 3.681 5.713 6.885 8.290 9.190 9.891 WQB1 (OUTFLOW)	
12 Combine 3, 9, 11 7.685 12.43 16.90 23.06 27.62 31.68 TOTAL PROPOSED SOU	
	JTH

Proj. file: Macro Model 2023-12-22.gpw

Friday, Dec 22, 2023

Hydrograph Summary Report

Hydraflow Hydrographs by Intelisolve v9.1

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description				
1	SCS Runoff	10.29	1	727	37,153				WS-EX-S				
2	SCS Runoff	3.128	1	726	10,553				WS-EX-E				
3	SCS Runoff	4.955	1	727	17,415				WS-PR-UNDET-S				
4	SCS Runoff	0.630	1	725	1,985				WS-PR-UNDET-E (TOTAL PROP. EA				
5	SCS Runoff	5.598	1	725	17,351				WS-PR-S-1 TO WQB #3				
6	Reservoir	0.568	1	775	11,732	5	160.61	9,617	WQB 3 (OUTFLOW)				
7	SCS Runoff	2.762	1	725	8,562				WS-PR-S-2 TO WQB #2				
8	Combine	2.777	1	725	20,294	6, 7			WQB 2 (TOTAL INFLOW)				
9	Reservoir	0.502	1	919	14,736	8	159.53	9,046	WQB 2 (OUTFLOW)				
10	SCS Runoff	10.69	1	727	36,899				WS-PR-S-3 TO WQB #1				
11	Reservoir	3.681	1	746	36,887	10	150.31	31,692	WQB1 (OUTFLOW)				
12	Combine	7.685	1	728	69,038	3, 9, 11			TOTAL PROPOSED SOUTH				
Mad	cro Model 202	23-12-22.	gpw		Return F	Period: 2 Ye	ar	Friday, Dec 22, 2023					

Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

= 10.29 cfs

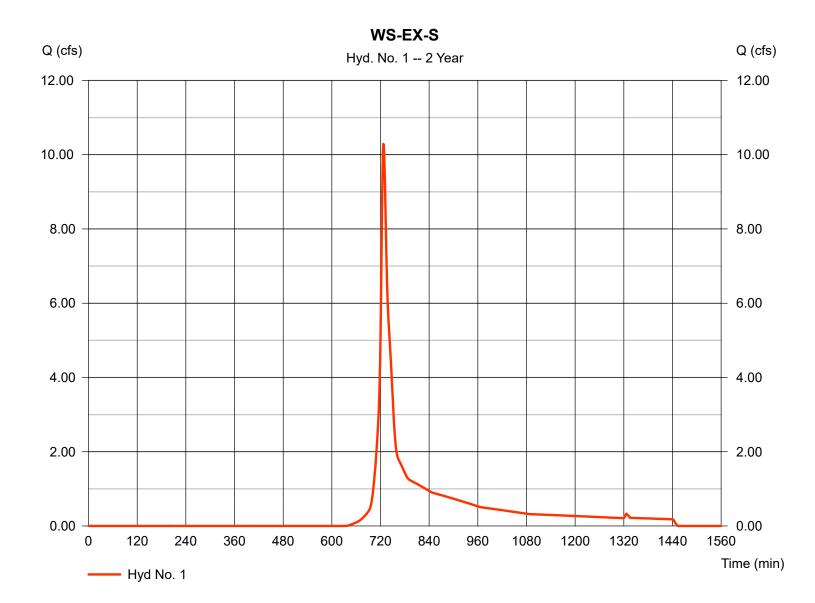
Hyd. No. 1

WS-EX-S

Hydrograph type = SCS Runoff Storm frequency = 2 yrsTime interval = 1 min Drainage area = 9.680 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 3.23 inStorm duration = 24 hrs

Time to peak = 727 min
Hyd. volume = 37,153 cuft
Curve number = 74
Hydraulic length = 0 ft
Time of conc. (Tc) = 9.10 min
Distribution = Type III
Shape factor = 484

Peak discharge



Hydraflow Hydrographs by Intelisolve v9.1

Hyd. No. 1

WS-EX-S

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>			
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.024 = 100.0 = 3.34 = 2.00		0.011 0.0 3.22 0.00		0.011 0.0 0.00 0.00					
Travel Time (min)	= 2.21	+	0.00	+	0.00	=	2.21			
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 678.00 = 3.29 = Unpave = 2.93	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00					
Travel Time (min)	= 3.86	+	0.00	+	0.00	=	3.86			
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 3.00 = 3.00 = 1.52 = 0.050 = 3.67 = 658.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0					
Travel Time (min)	= 2.98	+	0.00	+	0.00	=	2.98			
Total Travel Time, Tc										

Hydraflow Hydrographs by Intelisolve v9.1

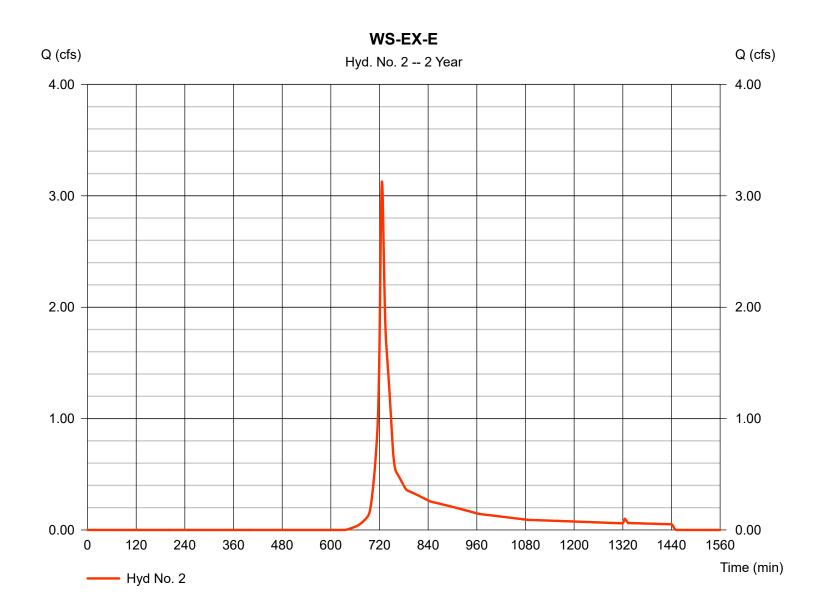
Friday, Dec 22, 2023

Hyd. No. 2

WS-EX-E

Hydrograph type = SCS Runoff Storm frequency = 2 yrs Time interval = 1 min Drainage area = 2.820 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 3.23 inStorm duration = 24 hrs

Peak discharge = 3.128 cfsTime to peak = 726 min Hyd. volume = 10,553 cuftCurve number = 74 Hydraulic length = 0 ftTime of conc. (Tc) $= 7.50 \, \text{min}$ Distribution = Type III = 484 Shape factor



Hydraflow Hydrographs by Intelisolve v9.1

Hyd. No. 2

WS-EX-E

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>			
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.024 = 100.0 = 3.34 = 2.00		0.011 0.0 3.22 0.00		0.011 0.0 0.00 0.00					
Travel Time (min)	= 2.21	+	0.00	+	0.00	=	2.21			
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 678.00 = 3.29 = Unpaved = 2.93		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00					
Travel Time (min)	= 3.86	+	0.00	+	0.00	=	3.86			
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 3.00 = 3.00 = 1.52 = 0.024 = 7.65 = 658.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0					
Travel Time (min)	= 1.43	+	0.00	+	0.00	=	1.43			
Total Travel Time, Tc										

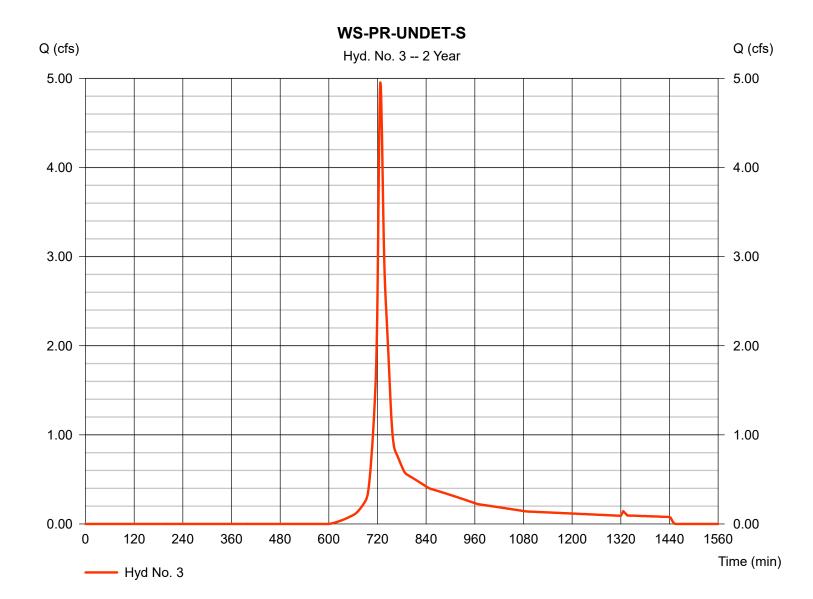
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 3

WS-PR-UNDET-S

Hydrograph type = SCS Runoff Peak discharge = 4.955 cfsStorm frequency Time to peak = 2 yrs= 727 min Time interval = 1 min Hyd. volume = 17,415 cuft Drainage area = 3.890 acCurve number = 77 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = USER Time of conc. (Tc) $= 10.00 \, \text{min}$ Distribution Total precip. = 3.23 in= Type III Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

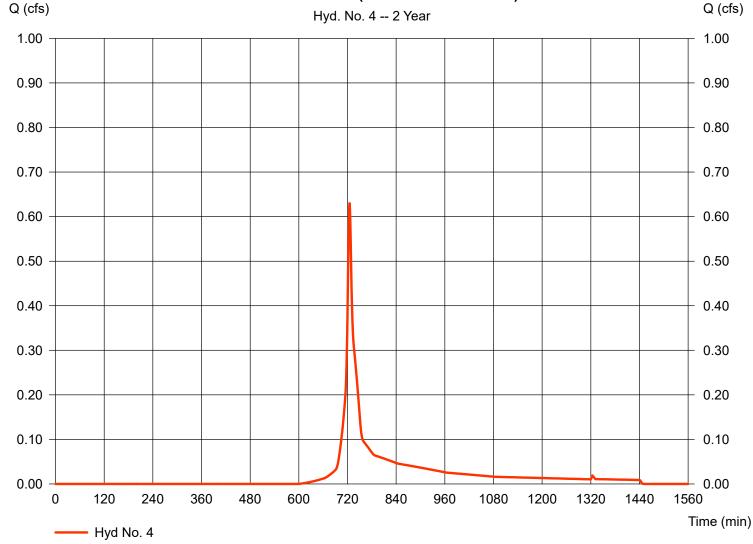
Hyd. No. 4

WS-PR-UNDET-E (TOTAL PROP. EAST)

Hydrograph type = SCS Runoff Storm frequency = 2 yrs Time interval = 1 min Drainage area = 0.430 acBasin Slope = 0.0 % Tc method = USER Total precip. = 3.23 inStorm duration = 24 hrs

Peak discharge = 0.630 cfsTime to peak = 725 min Hyd. volume = 1,985 cuft Curve number = 77 Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Distribution = Type III Shape factor = 484

WS-PR-UNDET-E (TOTAL PROP. EAST)



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

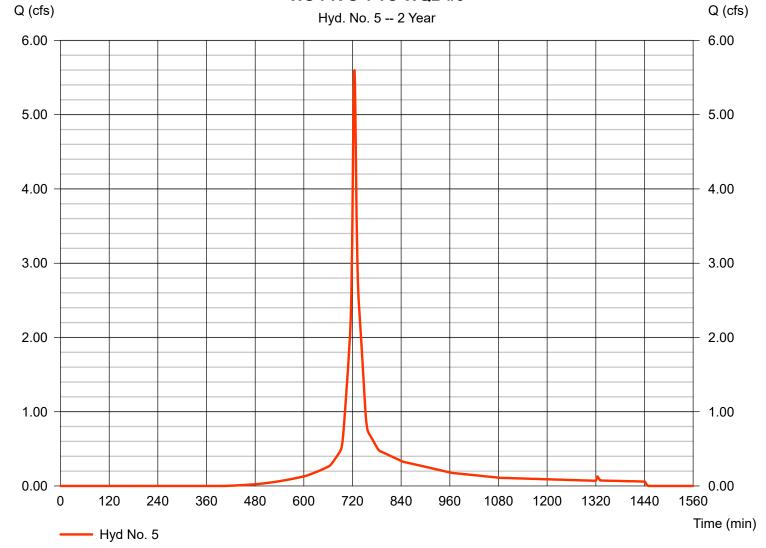
Hyd. No. 5

WS-PR-S-1 TO WQB #3

Hydrograph type = SCS Runoff Storm frequency = 2 yrsTime interval = 1 min Drainage area = 2.290 acBasin Slope = 0.0 % Tc method = USER Total precip. = 3.23 inStorm duration = 24 hrs

Peak discharge = 5.598 cfsTime to peak = 725 min Hyd. volume = 17,351 cuftCurve number = 88 Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Distribution = Type III Shape factor = 484

WS-PR-S-1 TO WQB #3



Hydraflow Hydrographs by Intelisolve v9.1

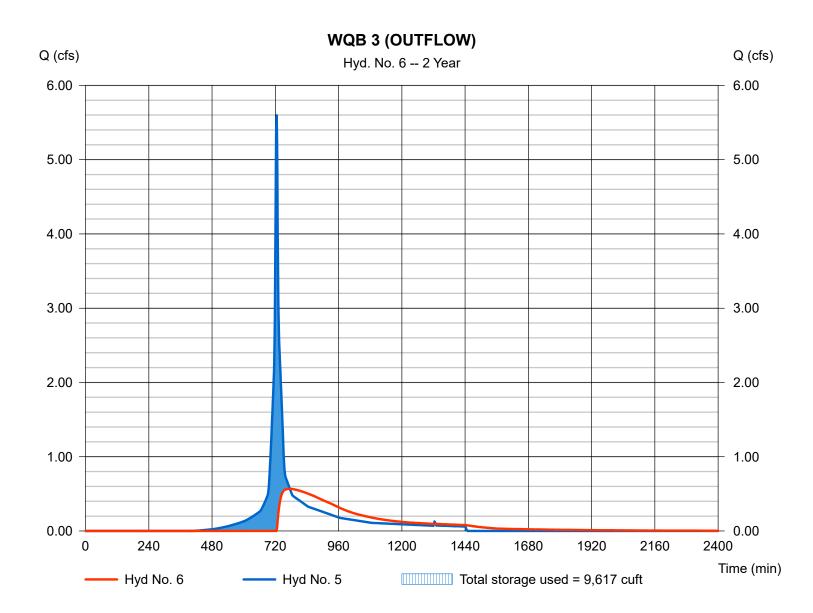
Friday, Dec 22, 2023

Hyd. No. 6

WQB 3 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 0.568 cfsStorm frequency Time to peak = 2 yrs = 775 min Time interval = 1 min Hyd. volume = 11,732 cuft Inflow hyd. No. = 5 - WS-PR-S-1 TO WQB #3 Max. Elevation $= 160.61 \, \text{ft}$ Reservoir name = WQB3 Max. Storage = 9,617 cuft

Storage Indication method used.



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Pond No. 3 - WQB3

Pond Data

Contours - User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 159.00 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	159.00	5,133	0	0
1.00	160.00	6,076	5,597	5,597
2.00	161.00	7,096	6,579	12,176
3.00	162.00	8,116	7,600	19,776
4.00	163.00	9,249	8,675	28,451
5.00	164.00	10,382	9,809	38,260
6.00	165.00	11,628	10,998	49,258
7.00	166.00	12,874	12,244	61,503

Culvert / Orifice Structures Weir Structures [A] [B] [C] [PrfRsr] [A] [B] [C] [D] 6.00 Rise (in) = 15.00 0.00 0.00 Crest Len (ft) = 11.33 0.00 0.00 0.00 Span (in) 0.00 = 15.00 6.00 0.00 0.00 Crest El. (ft) = 164.00 0.00 0.00 No. Barrels = 1 0 Weir Coeff. = 3.331 1 3.33 3.33 3.33 Invert El. (ft) = 159.00 160.00 0.00 0.00 Weir Type = Riser Length (ft) = 54.00 0.00 0.00 0.00 Multi-Stage = Yes No No No 0.00 Slope (%) = 0.00 0.00 n/a = .012 .013 .013 n/a N-Value Orifice Coeff. = 0.600.60 0.60 0.60 Exfil.(in/hr) = 0.000 (by Contour) Multi-Stage = n/a Yes TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	CIv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	159.00	0.00	0.00			0.00						0.00
1.00	5,597	160.00	0.00	0.00			0.00						0.00
2.00	12,176	161.00	0.83 ic	0.82 ic			0.00						0.82
3.00	19,776	162.00	1.25 ic	1.25 ic			0.00						1.25
4.00	28,451	163.00	1.59 ic	1.57 ic			0.00						1.57
5.00	38,260	164.00	1.83 ic	1.83 ic			0.00						1.83
6.00	49,258	165.00	13.35 oc	0.20 ic			13.15 s						13.35
7.00	61,503	166.00	14.75 oc	0.08 ic			14.63 s						14.71

Hydraflow Hydrographs by Intelisolve v9.1

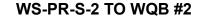
Friday, Dec 22, 2023

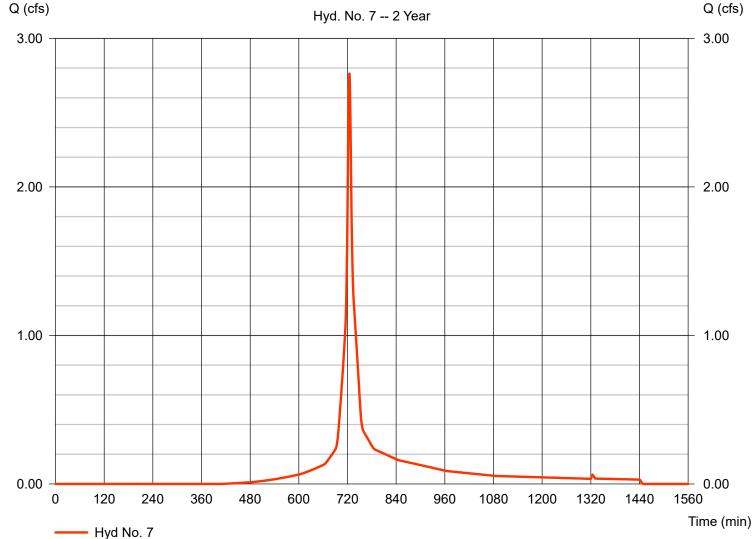
Hyd. No. 7

WS-PR-S-2 TO WQB #2

Hydrograph type = SCS Runoff Storm frequency = 2 yrsTime interval = 1 min Drainage area = 1.130 acBasin Slope = 0.0 % Tc method = USER Total precip. = 3.23 inStorm duration = 24 hrs

Peak discharge = 2.762 cfsTime to peak = 725 min Hyd. volume = 8,562 cuft Curve number = 88 Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Distribution = Type III = 484 Shape factor





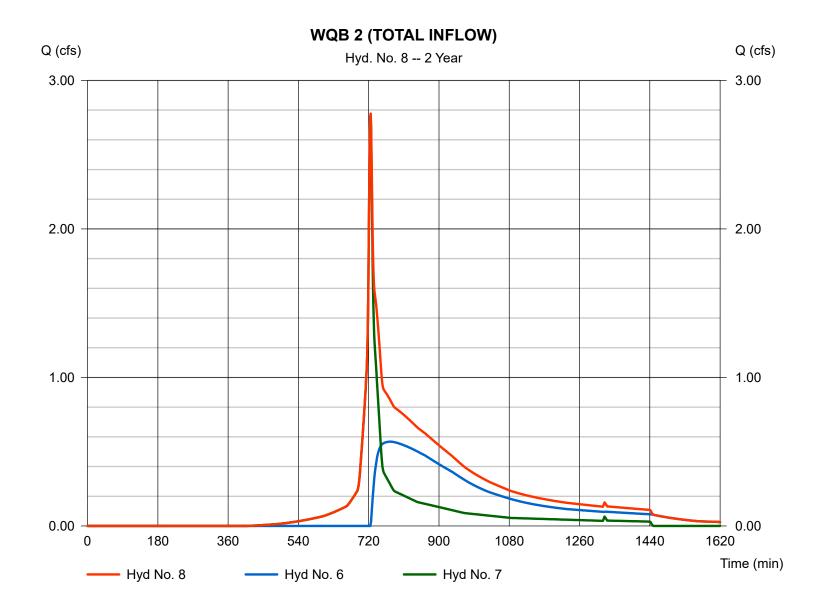
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 8

WQB 2 (TOTAL INFLOW)

Hydrograph type = Combine Storm frequency = 2 yrs Time interval = 1 min Inflow hyds. = 6, 7 Peak discharge = 2.777 cfs
Time to peak = 725 min
Hyd. volume = 20,294 cuft
Contrib. drain. area = 1.130 ac



= WQB2

Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

= 9,046 cuft

Hyd. No. 9

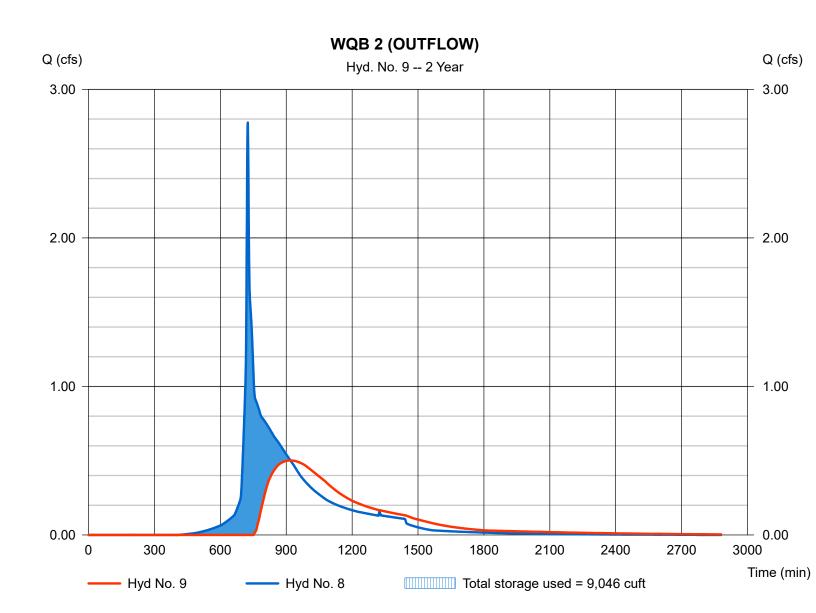
WQB 2 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 0.502 cfsStorm frequency Time to peak = 2 yrs = 919 min Time interval = 1 min Hyd. volume = 14,736 cuft= 8 - WQB 2 (TOTAL INFLOW) Inflow hyd. No. Max. Elevation = 159.53 ft

Max. Storage

Storage Indication method used.

Reservoir name



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Pond No. 2 - WQB2

Pond Data

Contours - User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 158.00 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	158.00	4,878	0	0
1.00	159.00	6,095	5,475	5,475
2.00	160.00	7,311	6,693	12,168
3.00	161.00	8,639	7,965	20,133
4.00	162.00	9,968	9,295	29,427
5.00	163.00	11,471	10,710	40,137
6.00	164.00	12,974	12,214	52,351
7.00	165.00	12,985	12,978	65,329

Culvert / Orifice Structures

Weir Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 15.00	6.00	0.00	0.00	Crest Len (ft)	= 11.33	0.00	0.00	0.00
Span (in)	= 15.00	6.00	0.00	0.00	Crest El. (ft)	= 162.50	0.00	0.00	0.00
No. Barrels	= 1	1	0	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 159.00	159.00	0.00	0.00	Weir Type	= Riser			
Length (ft)	= 36.00	0.00	0.00	0.00	Multi-Stage	= Yes	No	No	No
Slope (%)	= 0.00	0.00	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	Contour)		
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	CIv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	158.00	0.00	0.00			0.00						0.00
1.00	5,475	159.00	0.00	0.00			0.00						0.00
2.00	12,168	160.00	0.00	0.82 ic			0.00						0.82
3.00	20,133	161.00	0.00	1.25 ic			0.00						1.25
4.00	29,427	162.00	0.00	1.57 ic			0.00						1.57
5.00	40,137	163.00	10.32 oc	1.83 ic			10.32 s						12.15
6.00	52,351	164.00	12.34 ic	2.06 ic			12.31 s						14.37
7.00	65,329	165.00	13.69 ic	2.27 ic			13.65 s						15.91

Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

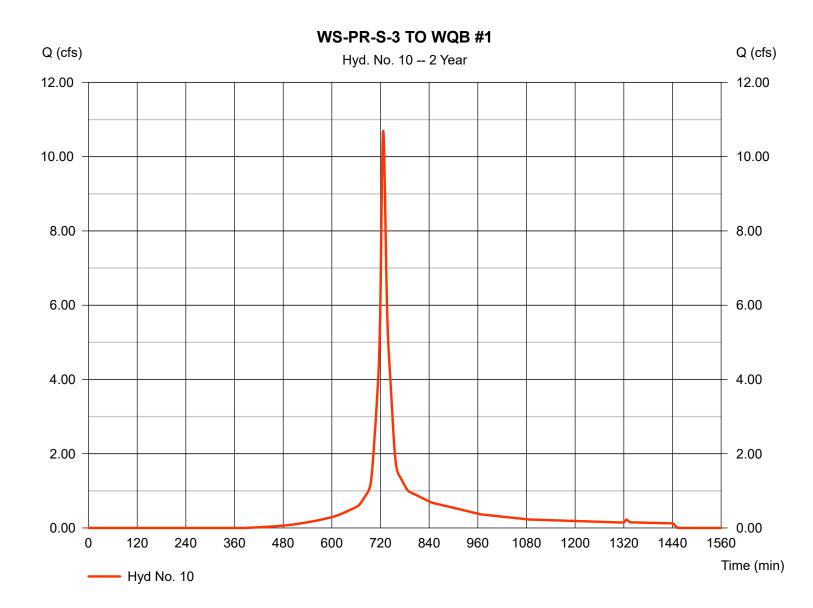
Hyd. No. 10

WS-PR-S-3 TO WQB #1

Hydrograph type = SCS Runoff Storm frequency = 2 yrsTime interval = 1 min Drainage area = 4.820 acBasin Slope = 0.0 % Tc method = USER Total precip. = 3.23 inStorm duration = 24 hrs

Peak discharge = 10.69 cfs
Time to peak = 727 min
Hyd. volume = 36,899 cuft
Curve number = 89
Hydraulic length = 0 ft

Time of conc. (Tc) = 10.00 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

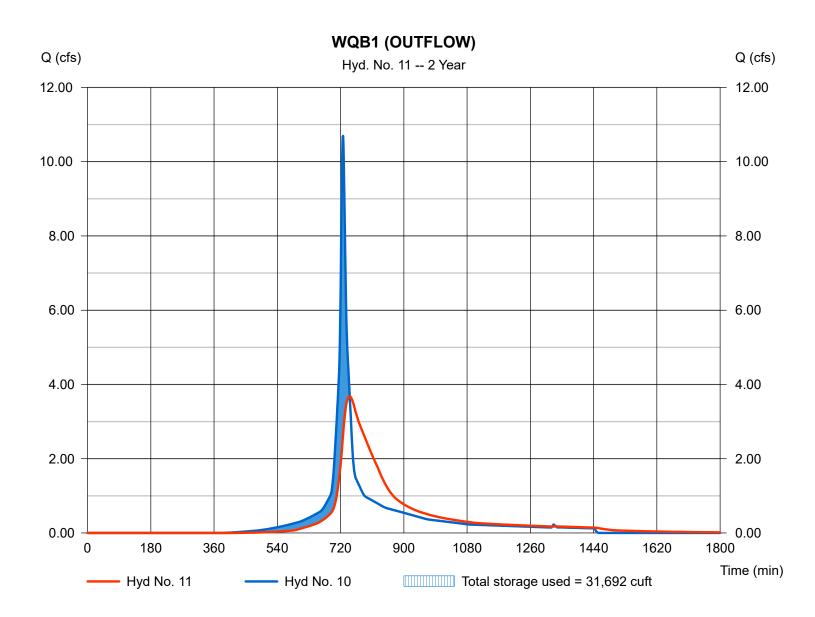
Hyd. No. 11

WQB1 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 3.681 cfsTime to peak Storm frequency = 2 yrs = 746 min Time interval = 1 min Hyd. volume = 36,887 cuft Inflow hyd. No. = 10 - WS-PR-S-3 TO WQB #1 Max. Elevation $= 150.31 \, \text{ft}$

Reservoir name = WQB1 Max. Storage = 31,692 cuft

Storage Indication method used. Wet pond routing start elevation = 149.00 ft.



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Pond No. 1 - WQB1

Pond Data

Contours - User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 146.00 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	146.00	5,069	0	0
1.00	147.00	6,017	5,536	5,536
2.00	148.00	7,066	6,534	12,070
3.00	149.00	8,217	7,634	19,703
4.00	150.00	9,469	8,835	28,538
5.00	151.00	10,801	10,127	38,664
6.00	152.00	12,249	11,516	50,181
7.00	153.00	13,656	12,945	63,126
8.00	154.00	15,120	14,380	77,506

Culvert / Orifice Structures

Weir Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 24.00	8.00	10.00	0.00	Crest Len (ft)	= 11.33	0.00	0.00	0.00
Span (in)	= 24.00	8.00	10.00	0.00	Crest El. (ft)	= 152.75	0.00	0.00	0.00
No. Barrels	= 1	2	1	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 148.00	149.00	150.00	0.00	Weir Type	= Riser			
Length (ft)	= 30.00	0.00	0.00	0.00	Multi-Stage	= Yes	No	No	No
Slope (%)	= 0.00	0.00	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	Contour)		
Multi-Stage	= n/a	Yes	Yes	No	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	CIv A cfs	CIv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	146.00	0.00	0.00	0.00		0.00						0.00
1.00	5,536	147.00	0.00	0.00	0.00		0.00						0.00
2.00	12,070	148.00	0.00	0.00	0.00		0.00						0.00
3.00	19,703	149.00	0.00	0.00	0.00		0.00						0.00
4.00	28,538	150.00	2.81 ic	2.74 ic	0.00		0.00						2.74
5.00	38,664	151.00	6.37 ic	4.34 ic	2.01 ic		0.00						6.34
6.00	50,181	152.00	8.79 ic	5.49 ic	3.30 ic		0.00						8.79
7.00	63,126	153.00	14.83 ic	5.87 ic	4.22 ic		4.72						14.81
8.00	77,506	154.00	33.11 ic	1.53 ic	1.20 ic		30.38 s						33.11

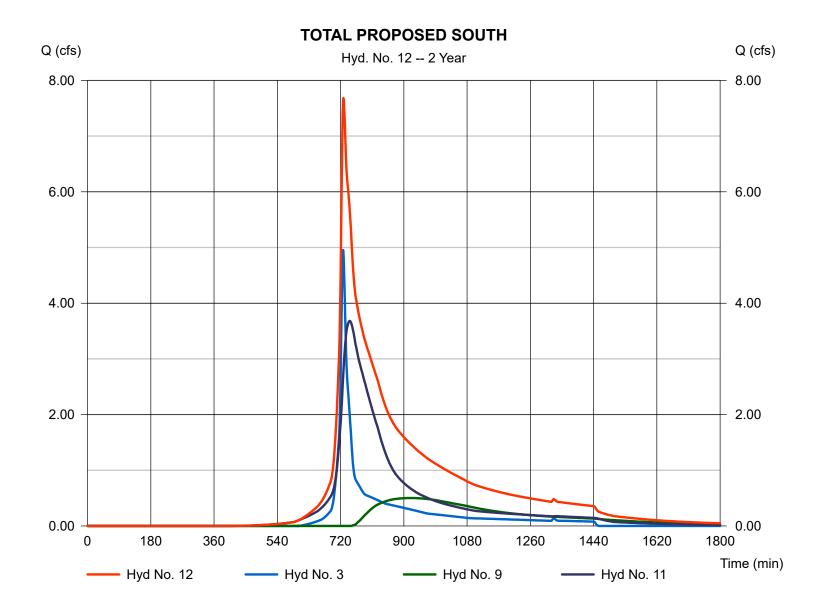
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 12

TOTAL PROPOSED SOUTH

Hydrograph type = Combine Storm frequency = 2 yrs Time interval = 1 min Inflow hyds. = 3, 9, 11 Peak discharge = 7.685 cfs Time to peak = 728 min Hyd. volume = 69,038 cuft Contrib. drain. area = 3.890 ac



Hydrograph Summary Report

Hydraflow Hydrographs by Intelisolve v9.1

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	SCS Runoff	18.43	1	727	63,946				WS-EX-S
2	SCS Runoff	5.587	1	726	18,163				WS-EX-E
3	SCS Runoff	8.420	1	727	28,938				WS-PR-UNDET-S
4	SCS Runoff	1.068	1	725	3,299				WS-PR-UNDET-E (TOTAL PROP. EA
5	SCS Runoff	8.217	1	724	25,791				WS-PR-S-1 TO WQB #3
6	Reservoir	0.948	1	765	20,173	5	161.26	14,130	WQB 3 (OUTFLOW)
7	SCS Runoff	4.054	1	724	12,727				WS-PR-S-2 TO WQB #2
8	Combine	4.598	1	725	32,899	6, 7			WQB 2 (TOTAL INFLOW)
9	Reservoir	0.851	1	926	27,332	8	160.06	12,653	WQB 2 (OUTFLOW)
10	SCS Runoff	15.52	1	727	54,339				WS-PR-S-3 TO WQB #1
11	Reservoir	5.713	1	744	54,327	10	150.80	36,647	WQB1 (OUTFLOW)
12	Combine	12.43	1	729	110,597	3, 9, 11			TOTAL PROPOSED SOUTH
Mad	cro Model 202	23-12-22.	gpw		Return P	eriod: 5 Ye	ar	Friday, Dec	22, 2023

Hydraflow Hydrographs by Intelisolve v9.1

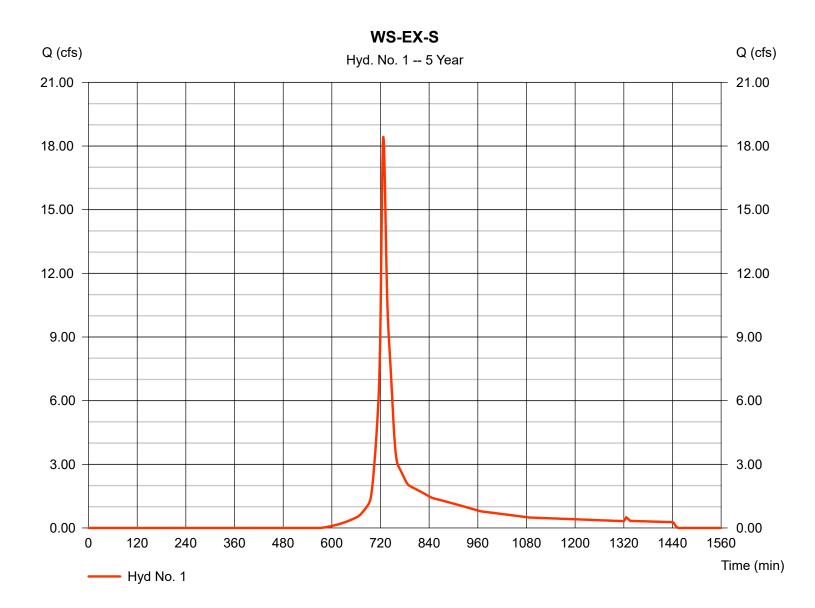
Friday, Dec 22, 2023

Hyd. No. 1

WS-EX-S

Hydrograph type = SCS Runoff Storm frequency = 5 yrsTime interval = 1 min Drainage area = 9.680 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 4.30 inStorm duration = 24 hrs

Peak discharge = 18.43 cfsTime to peak = 727 min Hyd. volume = 63,946 cuft Curve number = 74 Hydraulic length = 0 ftTime of conc. (Tc) $= 9.10 \, \text{min}$ Distribution = Type III = 484 Shape factor



Hydraflow Hydrographs by Intelisolve v9.1

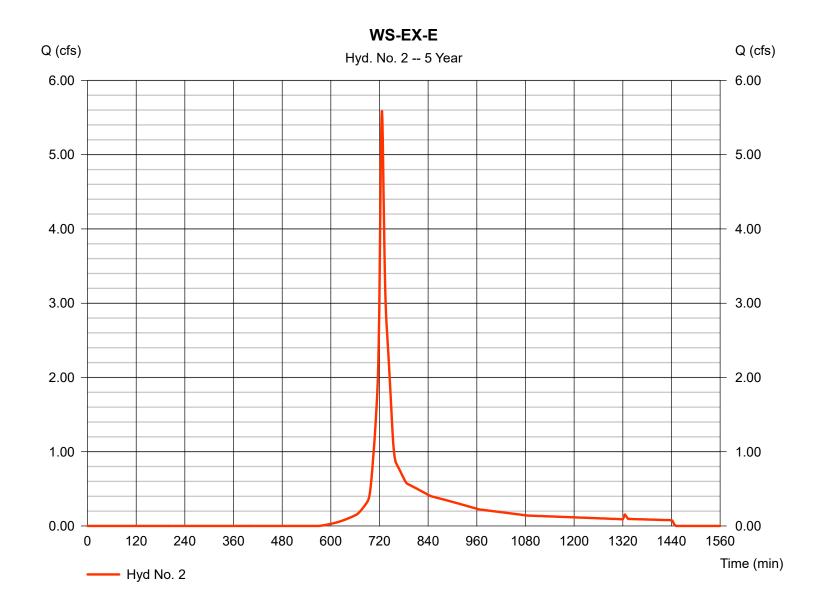
Friday, Dec 22, 2023

Hyd. No. 2

WS-EX-E

Hydrograph type = SCS Runoff Storm frequency = 5 yrsTime interval = 1 min Drainage area = 2.820 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 4.30 inStorm duration = 24 hrs

Peak discharge = 5.587 cfsTime to peak = 726 min Hyd. volume = 18,163 cuft Curve number = 74 Hydraulic length = 0 ftTime of conc. (Tc) $= 7.50 \, \text{min}$ Distribution = Type III Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 3

WS-PR-UNDET-S

Hydrograph type = SCS Runoff Storm frequency = 5 yrsTime interval = 1 min Drainage area = 3.890 acBasin Slope = 0.0 % Tc method = USER Total precip. = 4.30 inStorm duration = 24 hrs

Peak discharge = 8.420 cfs
Time to peak = 727 min
Hyd. volume = 28,938 cuft
Curve number = 77
Hydraulic length = 0 ft
Time of conc. (Tc) = 10.00 min

= Type III

= 484

Distribution

Shape factor

WS-PR-UNDET-S Q (cfs) Q (cfs) Hyd. No. 3 -- 5 Year 10.00 10.00 8.00 8.00 6.00 6.00 4.00 4.00 2.00 2.00 0.00 0.00 120 240 360 480 600 720 960 1080 1200 1320 1440 1560 0 840 Time (min) Hyd No. 3

Hydraflow Hydrographs by Intelisolve v9.1

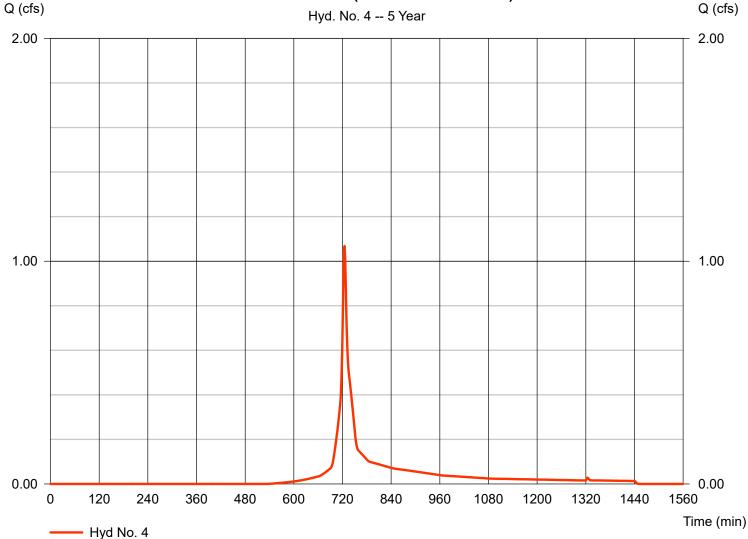
Friday, Dec 22, 2023

Hyd. No. 4

WS-PR-UNDET-E (TOTAL PROP. EAST)

Hydrograph type = SCS Runoff Peak discharge = 1.068 cfsStorm frequency Time to peak = 5 yrs= 725 min Time interval = 1 min Hyd. volume = 3,299 cuftDrainage area = 0.430 acCurve number = 77 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = USER Time of conc. (Tc) $= 5.00 \, \text{min}$ Distribution Total precip. = 4.30 in= Type III Storm duration = 24 hrs Shape factor = 484





Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 5

WS-PR-S-1 TO WQB #3

Hydrograph type = SCS Runoff Storm frequency = 5 yrsTime interval = 1 min Drainage area = 2.290 acBasin Slope = 0.0 % Tc method = USER Total precip. = 4.30 inStorm duration = 24 hrs

Hyd No. 5

= 8.217 cfsPeak discharge Time to peak = 724 min Hyd. volume = 25,791 cuftCurve number = 88 Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Distribution = Type III = 484 Shape factor

WS-PR-S-1 TO WQB #3 Q (cfs) Q (cfs) Hyd. No. 5 -- 5 Year 10.00 10.00 8.00 8.00 6.00 6.00 4.00 4.00 2.00 2.00 0.00 0.00 120 240 360 480 600 720 960 1080 1200 1320 1440 1560 0 840 Time (min)

Hydraflow Hydrographs by Intelisolve v9.1

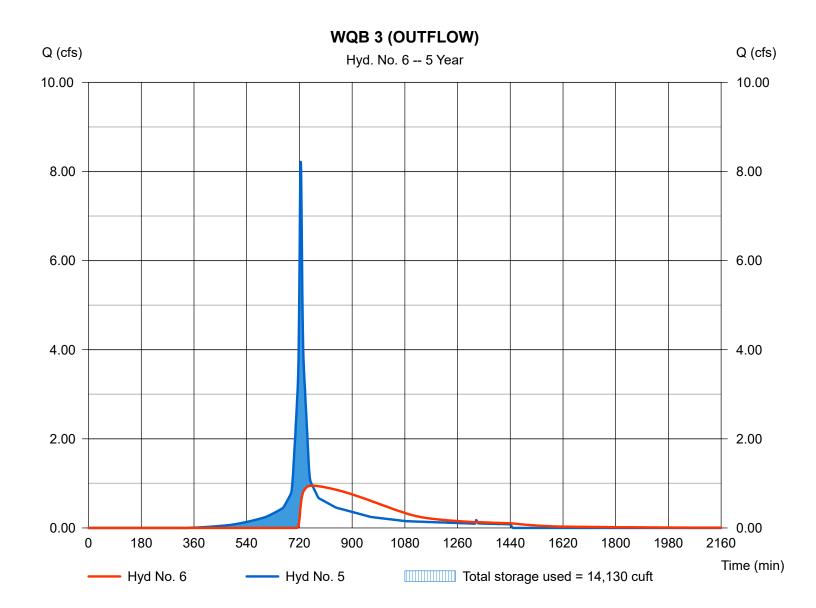
Friday, Dec 22, 2023

Hyd. No. 6

WQB 3 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 0.948 cfsStorm frequency Time to peak = 5 yrs $= 765 \, \text{min}$ Time interval = 1 min Hyd. volume = 20,173 cuftInflow hyd. No. = 5 - WS-PR-S-1 TO WQB #3 Max. Elevation = 161.26 ftReservoir name = WQB3 Max. Storage = 14,130 cuft

Storage Indication method used.



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

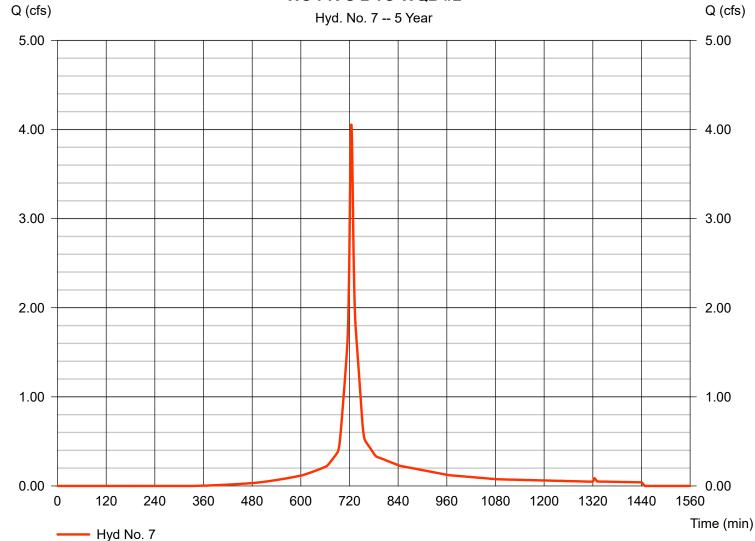
Hyd. No. 7

WS-PR-S-2 TO WQB #2

Hydrograph type = SCS Runoff Storm frequency = 5 yrsTime interval = 1 min Drainage area = 1.130 acBasin Slope = 0.0 % Tc method = USER Total precip. = 4.30 inStorm duration = 24 hrs

= 4.054 cfsPeak discharge Time to peak = 724 min Hyd. volume = 12,727 cuftCurve number = 88 Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Distribution = Type III Shape factor = 484

WS-PR-S-2 TO WQB #2



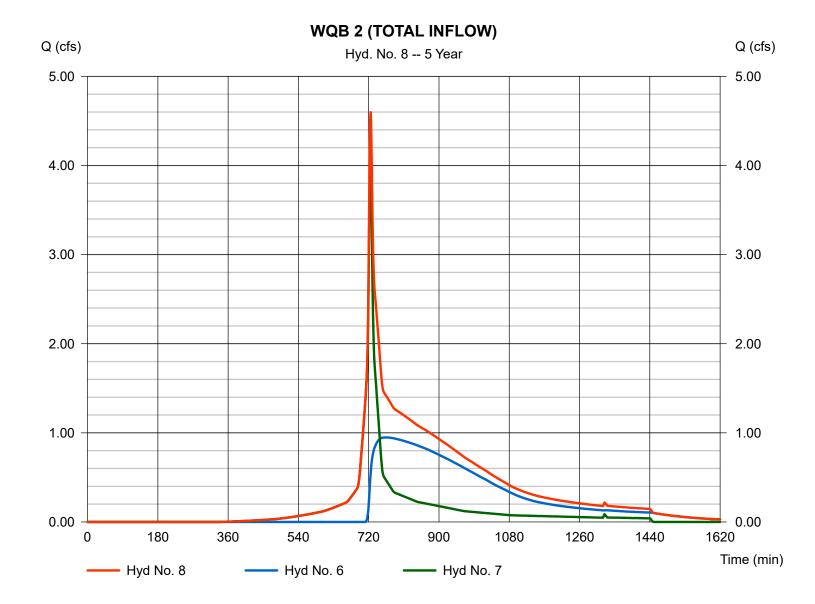
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 8

WQB 2 (TOTAL INFLOW)

Hydrograph type = Combine Storm frequency = 5 yrs Time interval = 1 min Inflow hyds. = 6, 7 Peak discharge = 4.598 cfs
Time to peak = 725 min
Hyd. volume = 32,899 cuft
Contrib. drain. area = 1.130 ac



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

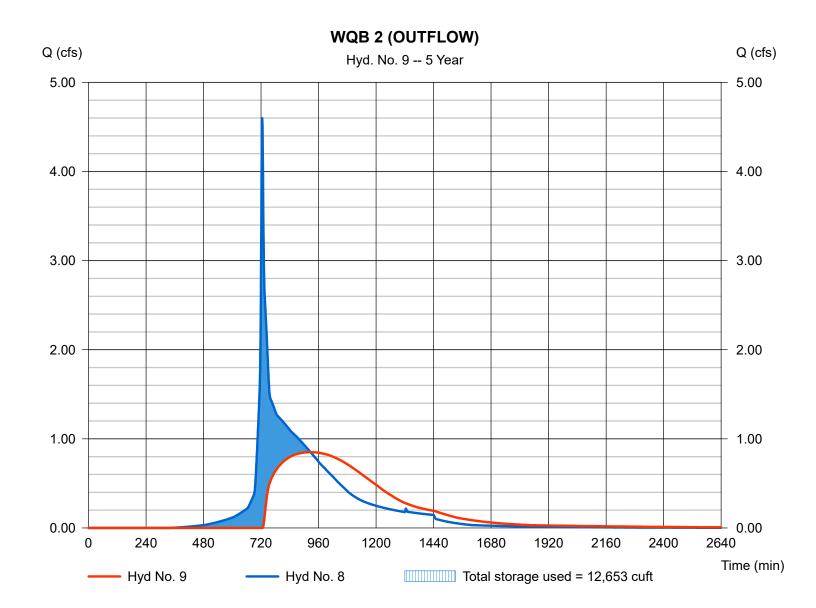
Hyd. No. 9

WQB 2 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 0.851 cfsTime to peak Storm frequency = 5 yrs= 926 min Time interval = 1 min Hyd. volume = 27,332 cuft= 8 - WQB 2 (TOTAL INFLOW) Inflow hyd. No. Max. Elevation = 160.06 ft

Reservoir name = WQB2 Max. Storage = 12,653 cuft

Storage Indication method used.



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

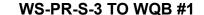
Hyd. No. 10

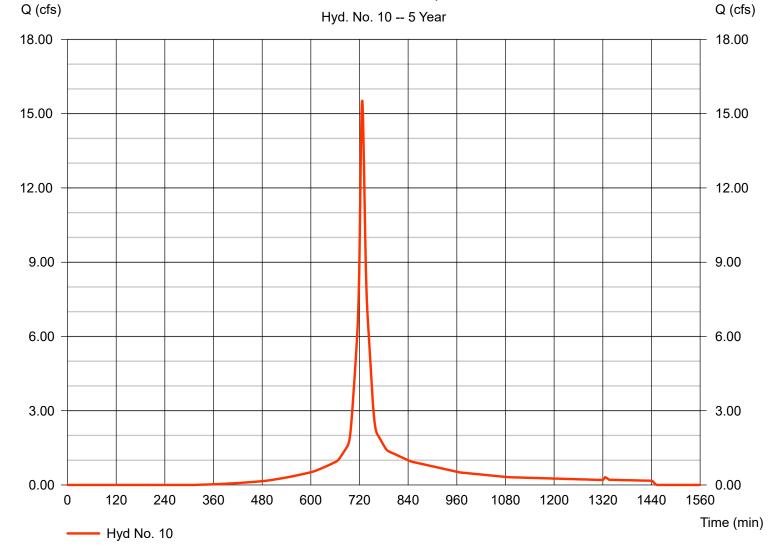
WS-PR-S-3 TO WQB #1

Hydrograph type = SCS Runoff Storm frequency = 5 yrsTime interval = 1 min Drainage area = 4.820 acBasin Slope = 0.0 % Tc method = USER Total precip. = 4.30 inStorm duration = 24 hrs

Peak discharge = 15.52 cfs
Time to peak = 727 min
Hyd. volume = 54,339 cuft
Curve number = 89
Hydraulic length = 0 ft
Time of conc. (Tc) = 10.00 min
Distribution = Type III

Shape factor = 484





Hydraflow Hydrographs by Intelisolve v9.1

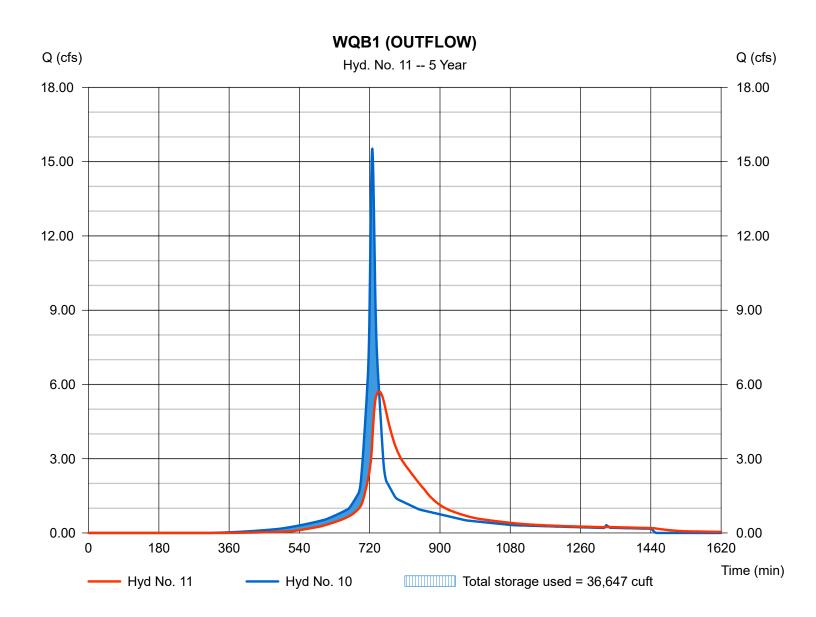
Friday, Dec 22, 2023

Hyd. No. 11

WQB1 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 5.713 cfsTime to peak Storm frequency = 5 yrs= 744 min Time interval = 1 min Hyd. volume = 54,327 cuftInflow hyd. No. = 10 - WS-PR-S-3 TO WQB #1 Max. Elevation = 150.80 ftReservoir name = WQB1 Max. Storage = 36,647 cuft

Storage Indication method used. Wet pond routing start elevation = 149.00 ft.



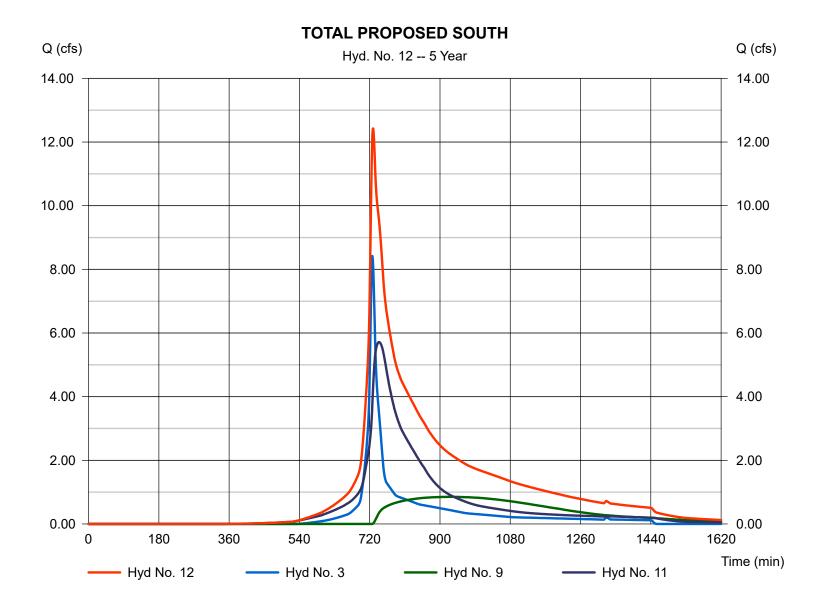
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 12

TOTAL PROPOSED SOUTH

Hydrograph type = Combine Storm frequency = 5 yrs Time interval = 1 min Inflow hyds. = 3, 9, 11 Peak discharge = 12.43 cfs Time to peak = 729 min Hyd. volume = 110,597 cuft Contrib. drain. area = 3.890 ac



Hydrograph Summary Report

Hydraflow Hydrographs by Intelisolve v9.1

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	SCS Runoff	25.42	1	727	87,300				WS-EX-S
2	SCS Runoff	7.698	1	726	24,797				WS-EX-E
3	SCS Runoff	11.34	1	727	38,817				WS-PR-UNDET-S
4	SCS Runoff	1.437	1	725	4,425				WS-PR-UNDET-E (TOTAL PROP. EA
5	SCS Runoff	10.31	1	724	32,675				WS-PR-S-1 TO WQB #3
6	Reservoir	1.166	1	765	27,056	5	161.77	18,044	WQB 3 (OUTFLOW)
7	SCS Runoff	5.088	1	724	16,123				WS-PR-S-2 TO WQB #2
8	Combine	5.901	1	725	43,180	6, 7			WQB 2 (TOTAL INFLOW)
9	Reservoir	1.047	1	947	37,606	8	160.48	15,964	WQB 2 (OUTFLOW)
10	SCS Runoff	19.35	1	727	68,511				WS-PR-S-3 TO WQB #1
11	Reservoir	6.885	1	745	68,499	10	151.19	40,839	WQB1 (OUTFLOW)
12	Combine	16.90	1	728	144,923	3, 9, 11			TOTAL PROPOSED SOUTH
Mad	cro Model 202	23-12-22.	gpw		Return P	Period: 10 Y	ear	Friday, Dec	22, 2023

Hydraflow Hydrographs by Intelisolve v9.1

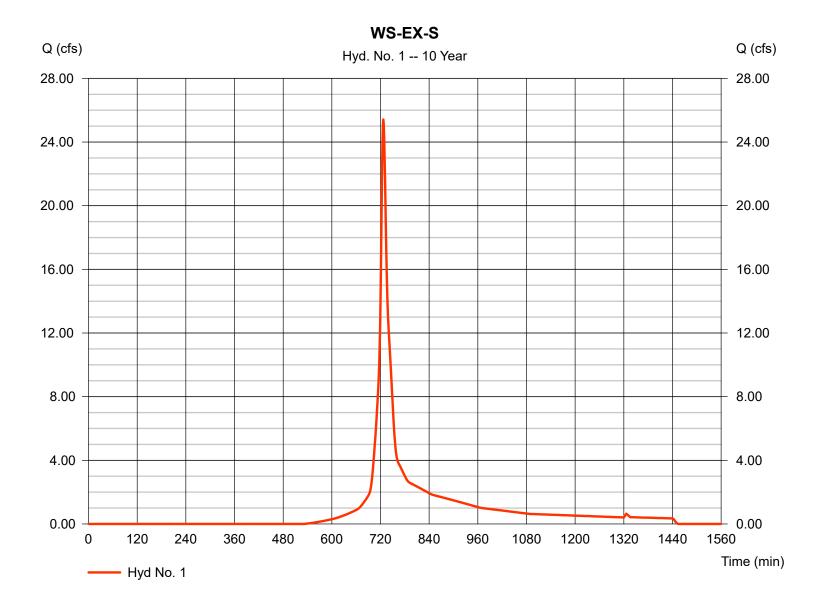
Friday, Dec 22, 2023

Hyd. No. 1

WS-EX-S

Hydrograph type = SCS Runoff Storm frequency = 10 yrsTime interval = 1 min Drainage area = 9.680 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 5.15 inStorm duration = 24 hrs

Peak discharge = 25.42 cfsTime to peak = 727 min Hyd. volume = 87,300 cuftCurve number = 74 Hydraulic length = 0 ftTime of conc. (Tc) $= 9.10 \, \text{min}$ Distribution = Type III Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.1

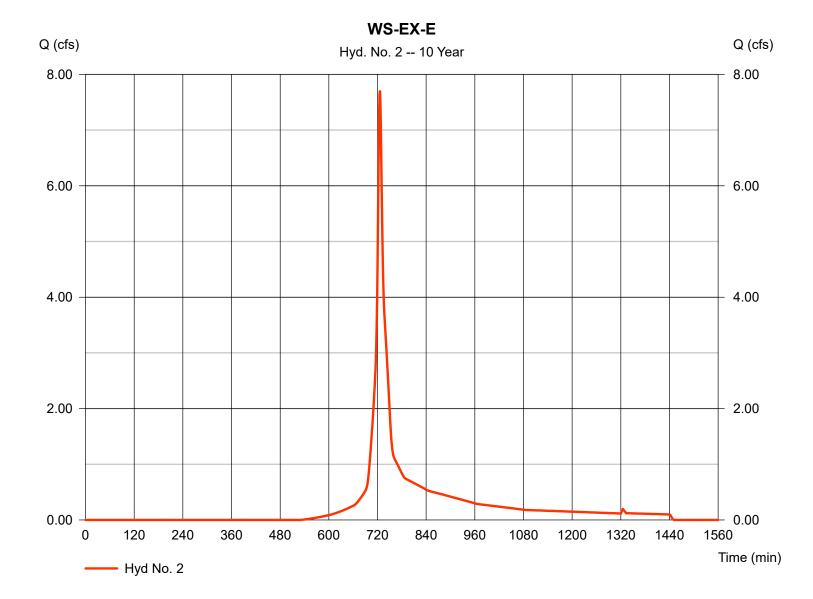
Friday, Dec 22, 2023

Hyd. No. 2

WS-EX-E

Hydrograph type = SCS Runoff Storm frequency = 10 yrsTime interval = 1 min Drainage area = 2.820 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 5.15 inStorm duration = 24 hrs

Peak discharge = 7.698 cfsTime to peak = 726 min Hyd. volume = 24,797 cuftCurve number = 74 Hydraulic length = 0 ftTime of conc. (Tc) $= 7.50 \, \text{min}$ Distribution = Type III = 484 Shape factor



Friday, Dec 22, 2023

Hydrograph Report

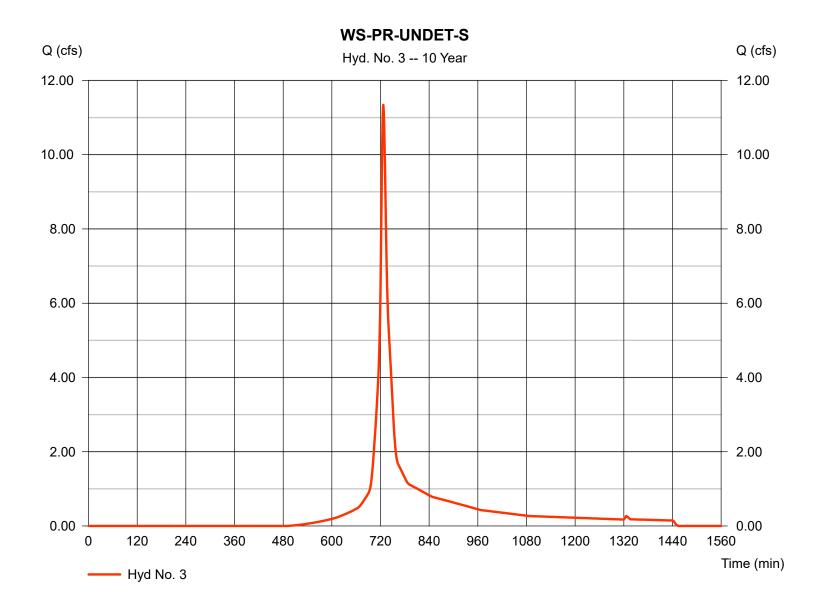
Hydraflow Hydrographs by Intelisolve v9.1

Blisolve v9.1

Hyd. No. 3

WS-PR-UNDET-S

Hydrograph type = SCS Runoff Peak discharge = 11.34 cfsStorm frequency Time to peak = 10 yrs= 727 min Time interval = 1 min Hyd. volume = 38,817 cuft Drainage area = 3.890 acCurve number = 77 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = USER Time of conc. (Tc) $= 10.00 \, \text{min}$ Distribution Total precip. = 5.15 in= Type III Storm duration = 484 = 24 hrs Shape factor



Hydraflow Hydrographs by Intelisolve v9.1

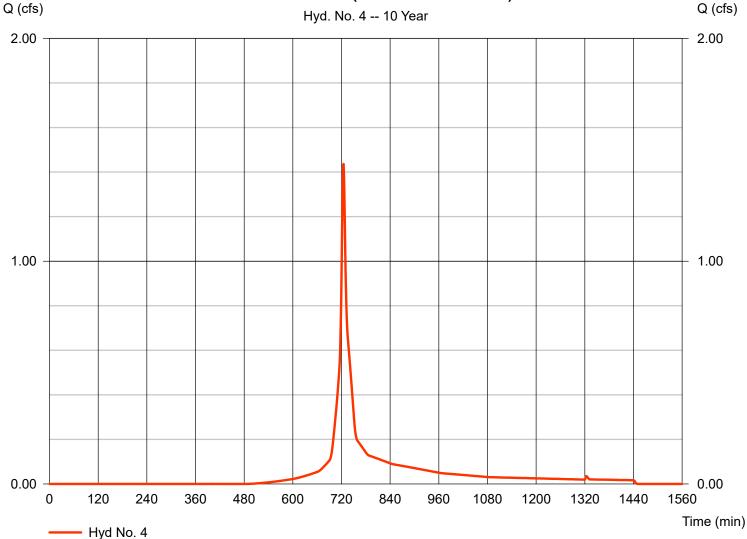
Friday, Dec 22, 2023

Hyd. No. 4

WS-PR-UNDET-E (TOTAL PROP. EAST)

= SCS Runoff Hydrograph type Peak discharge = 1.437 cfsStorm frequency Time to peak = 10 yrs $= 725 \, \text{min}$ Time interval = 1 min Hyd. volume = 4,425 cuftDrainage area = 0.430 acCurve number = 77 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = USER Time of conc. (Tc) $= 5.00 \, \text{min}$ Distribution Total precip. = 5.15 in= Type III Storm duration = 24 hrs Shape factor = 484





Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

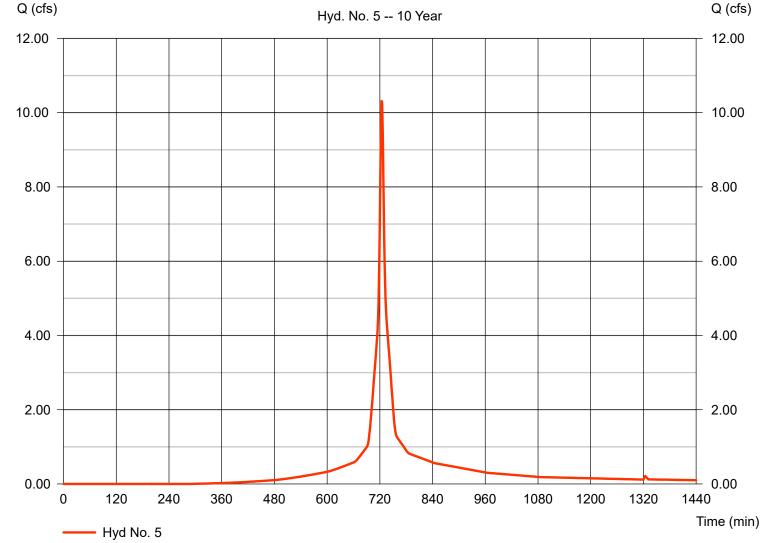
Hyd. No. 5

WS-PR-S-1 TO WQB #3

Hydrograph type = SCS Runoff Storm frequency = 10 yrsTime interval = 1 min Drainage area = 2.290 acBasin Slope = 0.0 % Tc method = USER Total precip. = 5.15 inStorm duration = 24 hrs

Peak discharge = 10.31 cfsTime to peak = 724 min Hyd. volume = 32,675 cuftCurve number = 88 Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Distribution = Type III = 484 Shape factor

WS-PR-S-1 TO WQB #3



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

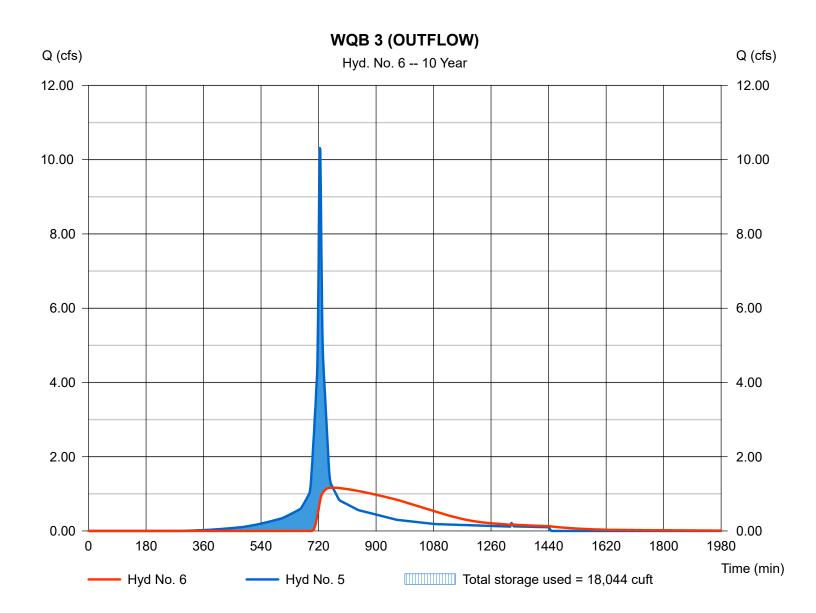
Hyd. No. 6

WQB 3 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 1.166 cfsTime to peak Storm frequency = 10 yrs $= 765 \, \text{min}$ Time interval = 1 min Hyd. volume = 27,056 cuftInflow hyd. No. = 5 - WS-PR-S-1 TO WQB #3 Max. Elevation = 161.77 ft

Reservoir name = WQB3 Max. Storage = 18,044 cuft

Storage Indication method used.



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

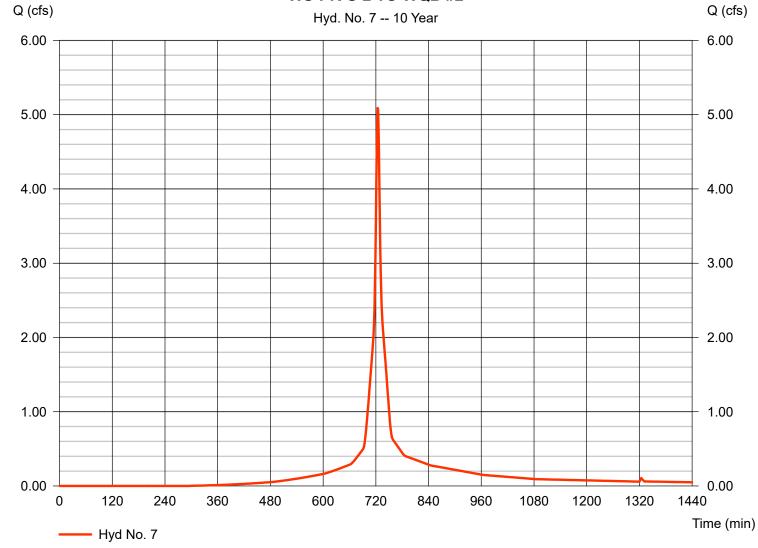
Hyd. No. 7

WS-PR-S-2 TO WQB #2

= SCS Runoff Hydrograph type Storm frequency = 10 yrsTime interval = 1 min Drainage area = 1.130 acBasin Slope = 0.0 % Tc method = USER Total precip. = 5.15 inStorm duration = 24 hrs

Peak discharge = 5.088 cfsTime to peak = 724 min Hyd. volume = 16,123 cuft Curve number = 88 Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Distribution = Type III Shape factor = 484

WS-PR-S-2 TO WQB #2



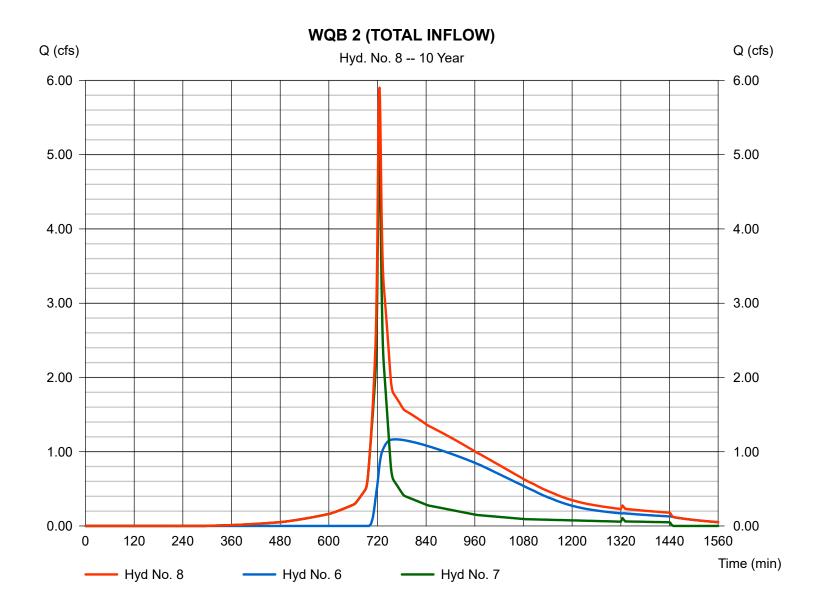
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 8

WQB 2 (TOTAL INFLOW)

Hydrograph type = Combine Storm frequency = 10 yrs Time interval = 1 min Inflow hyds. = 6, 7 Peak discharge = 5.901 cfs
Time to peak = 725 min
Hyd. volume = 43,180 cuft
Contrib. drain. area = 1.130 ac



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

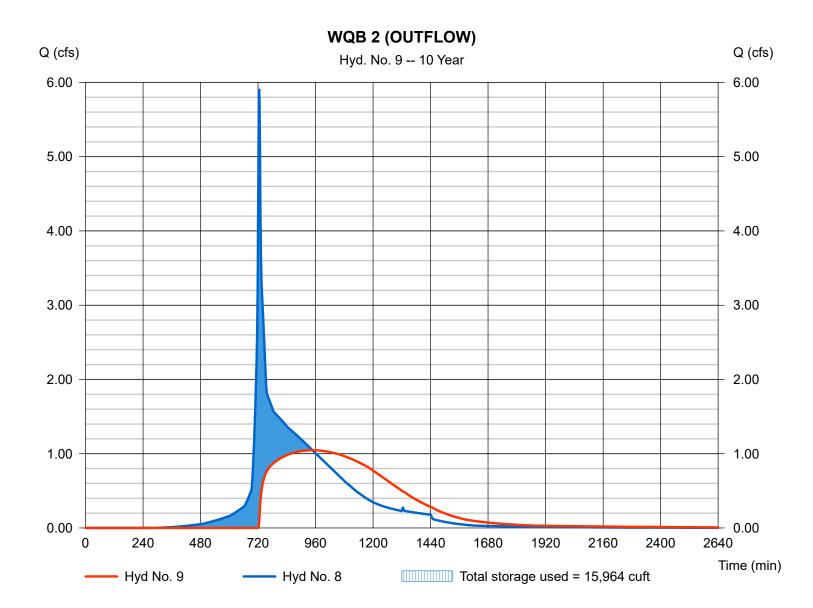
Hyd. No. 9

WQB 2 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 1.047 cfsTime to peak Storm frequency = 10 yrs= 947 min Time interval = 1 min Hyd. volume = 37,606 cuft= 8 - WQB 2 (TOTAL INFLOW) Inflow hyd. No. Max. Elevation = 160.48 ft

Reservoir name = WQB2 Max. Storage = 15,964 cuft

Storage Indication method used.



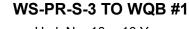
Hydraflow Hydrographs by Intelisolve v9.1

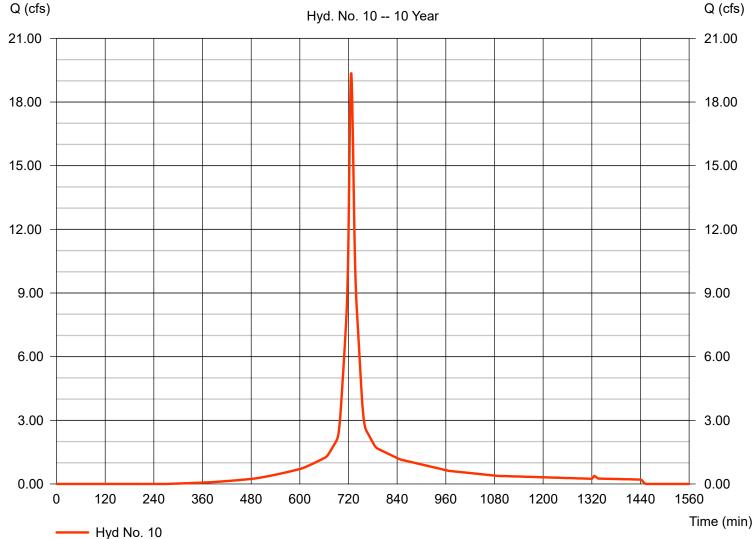
Friday, Dec 22, 2023

Hyd. No. 10

WS-PR-S-3 TO WQB #1

Hydrograph type = SCS Runoff Peak discharge = 19.35 cfsStorm frequency Time to peak = 10 yrs= 727 min Time interval = 1 min Hyd. volume = 68,511 cuftDrainage area = 4.820 acCurve number = 89 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = USER Time of conc. (Tc) $= 10.00 \, \text{min}$ Distribution Total precip. = 5.15 in= Type III Storm duration = 24 hrs Shape factor = 484





Hydraflow Hydrographs by Intelisolve v9.1

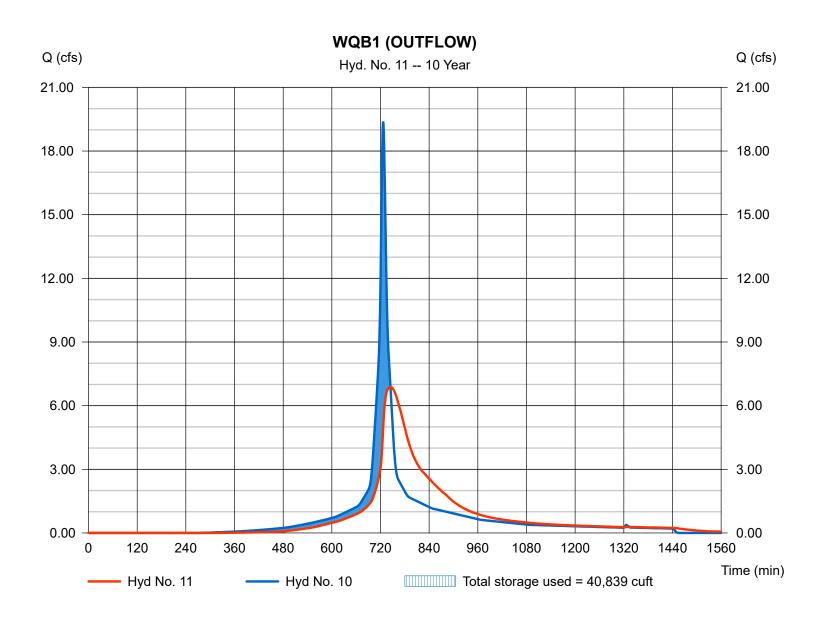
Friday, Dec 22, 2023

Hyd. No. 11

WQB1 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 6.885 cfsTime to peak Storm frequency = 10 yrs $= 745 \, \text{min}$ Time interval = 1 min Hyd. volume = 68,499 cuftInflow hyd. No. = 10 - WS-PR-S-3 TO WQB #1 Max. Elevation = 151.19 ft Reservoir name = WQB1 Max. Storage = 40,839 cuft

Storage Indication method used. Wet pond routing start elevation = 149.00 ft.



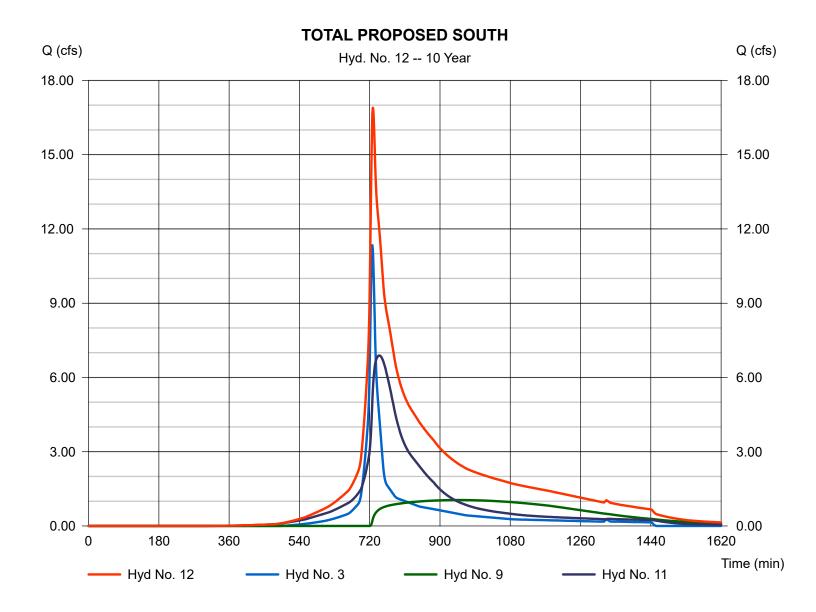
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 12

TOTAL PROPOSED SOUTH

Hydrograph type = Combine Storm frequency = 10 yrs Time interval = 1 min Inflow hyds. = 3, 9, 11 Peak discharge = 16.90 cfs
Time to peak = 728 min
Hyd. volume = 144,923 cuft
Contrib. drain. area = 3.890 ac



Hydrograph Summary Report

Hydraflow Hydrographs by Intelisolve v9.1

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	SCS Runoff	36.18	1	727	123,829				WS-EX-S
2	SCS Runoff	10.94	1	726	35,172				WS-EX-E
3	SCS Runoff	15.77	1	727	54,092				WS-PR-UNDET-S
4	SCS Runoff	1.996	1	725	6,166				WS-PR-UNDET-E (TOTAL PROP. EA
5	SCS Runoff	13.38	1	724	42,964				WS-PR-S-1 TO WQB #3
ŝ	Reservoir	1.410	1	768	37,346	5	162.48	23,896	WQB 3 (OUTFLOW)
7	SCS Runoff	6.603	1	724	21,201				WS-PR-S-2 TO WQB #2
3	Combine	7.653	1	725	58,547	6, 7			WQB 2 (TOTAL INFLOW)
9	Reservoir	1.290	1	968	52,963	8	161.11	21,168	WQB 2 (OUTFLOW)
10	SCS Runoff	24.98	1	726	89,646				WS-PR-S-3 TO WQB #1
11	Reservoir	8.290	1	746	89,633	10	151.77	47,478	WQB1 (OUTFLOW)
12	Combine	23.06	1	728	196,688	3, 9, 11			TOTAL PROPOSED SOUTH
	cro Model 202	23-12-22	apw		Return P	Period: 25 Y	rear	Friday, Dec	22. 2023

Hydraflow Hydrographs by Intelisolve v9.1

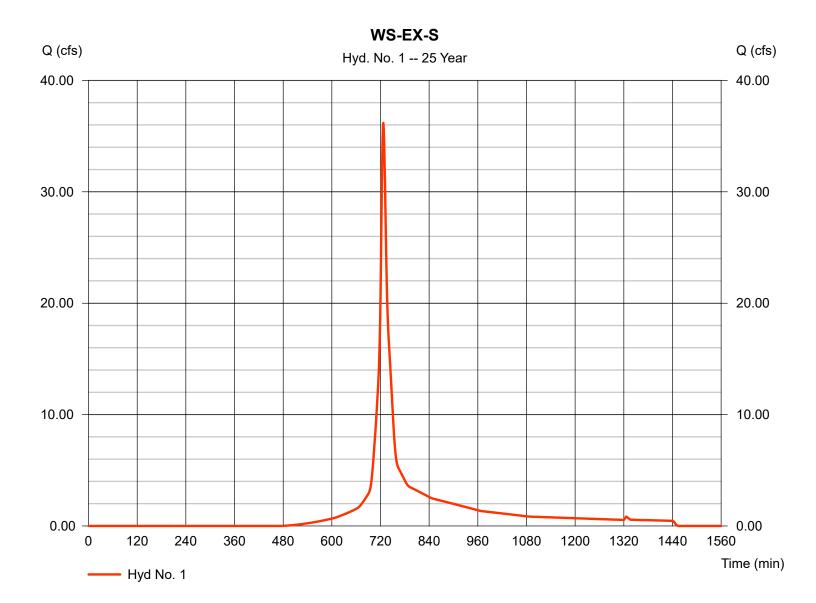
Friday, Dec 22, 2023

Hyd. No. 1

WS-EX-S

Hydrograph type = SCS Runoff Storm frequency = 25 yrs Time interval = 1 min Drainage area = 9.680 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 6.40 inStorm duration = 24 hrs

Peak discharge = 36.18 cfsTime to peak = 727 min Hyd. volume = 123,829 cuft Curve number = 74 Hydraulic length = 0 ftTime of conc. (Tc) $= 9.10 \, \text{min}$ Distribution = Type III = 484 Shape factor



Hydraflow Hydrographs by Intelisolve v9.1

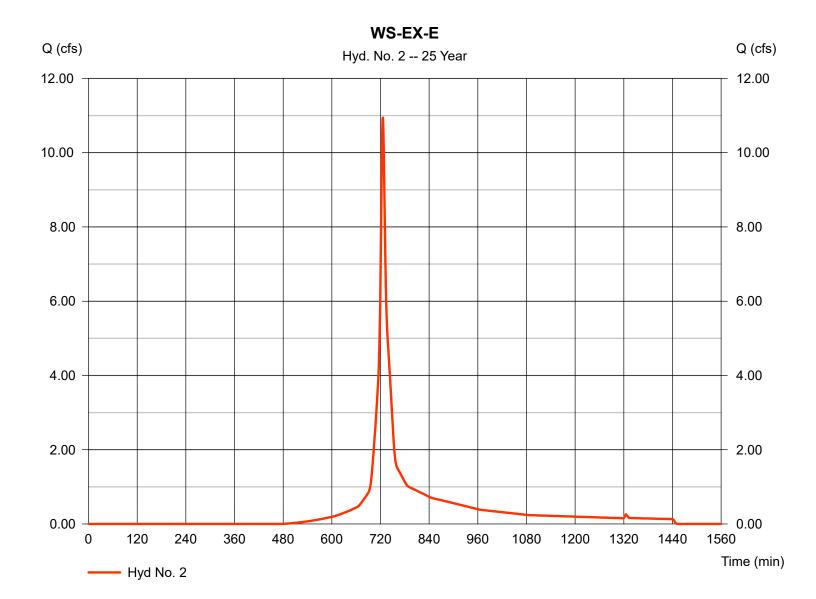
Friday, Dec 22, 2023

Hyd. No. 2

WS-EX-E

Hydrograph type = SCS Runoff Storm frequency = 25 yrs Time interval = 1 min Drainage area = 2.820 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 6.40 inStorm duration = 24 hrs

Peak discharge = 10.94 cfsTime to peak = 726 min Hyd. volume = 35,172 cuft Curve number = 74 Hydraulic length = 0 ftTime of conc. (Tc) $= 7.50 \, \text{min}$ Distribution = Type III = 484 Shape factor



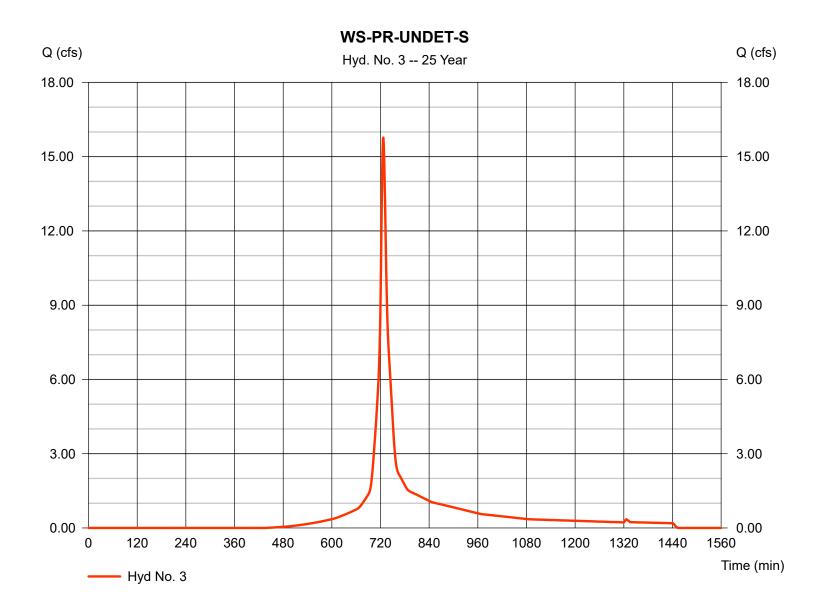
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 3

WS-PR-UNDET-S

Hydrograph type = SCS Runoff Peak discharge = 15.77 cfsStorm frequency Time to peak = 25 yrs = 727 min Time interval = 1 min Hyd. volume = 54,092 cuftDrainage area = 3.890 acCurve number = 77 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = USER Time of conc. (Tc) $= 10.00 \, \text{min}$ Distribution Total precip. = 6.40 in= Type III Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.1

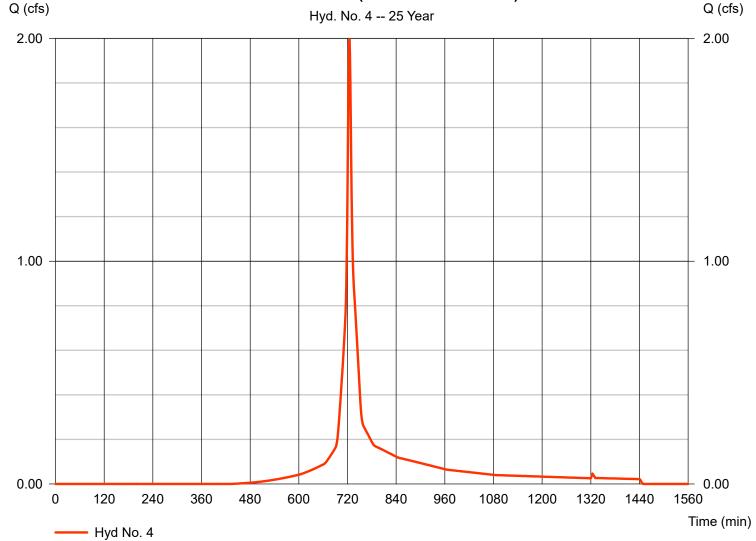
Friday, Dec 22, 2023

Hyd. No. 4

WS-PR-UNDET-E (TOTAL PROP. EAST)

= SCS Runoff Hydrograph type Peak discharge = 1.996 cfsStorm frequency Time to peak = 25 yrs $= 725 \, \text{min}$ Time interval = 1 min Hyd. volume = 6,166 cuftDrainage area = 0.430 acCurve number = 77 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = USER Time of conc. (Tc) $= 5.00 \, \text{min}$ Distribution Total precip. = 6.40 in= Type III Storm duration = 24 hrs Shape factor = 484





Hydraflow Hydrographs by Intelisolve v9.1

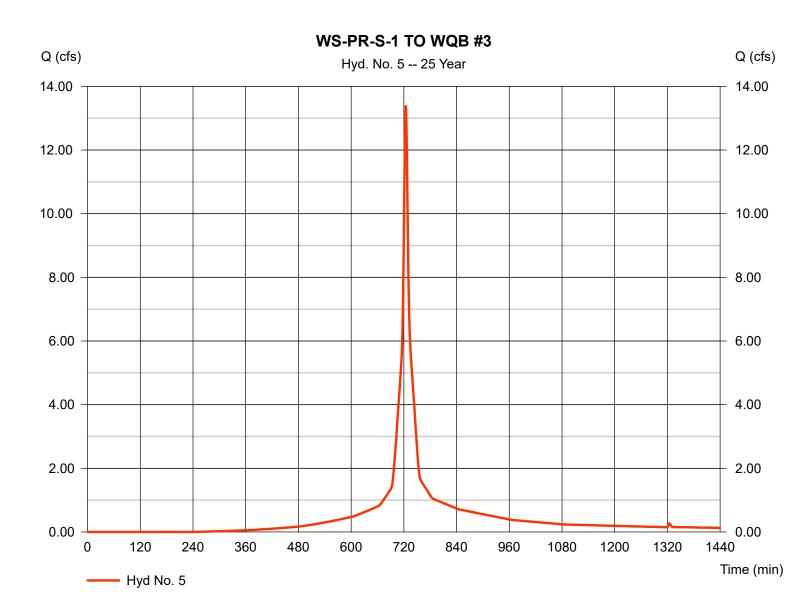
Friday, Dec 22, 2023

Hyd. No. 5

WS-PR-S-1 TO WQB #3

Hydrograph type = SCS Runoff Storm frequency = 25 yrs Time interval = 1 min Drainage area = 2.290 acBasin Slope = 0.0 % Tc method = USER Total precip. = 6.40 inStorm duration = 24 hrs

Peak discharge = 13.38 cfs= 724 min Time to peak Hyd. volume = 42,964 cuft Curve number = 88 Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Distribution = Type III = 484 Shape factor



= WQB3

Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

= 23,896 cuft

Hyd. No. 6

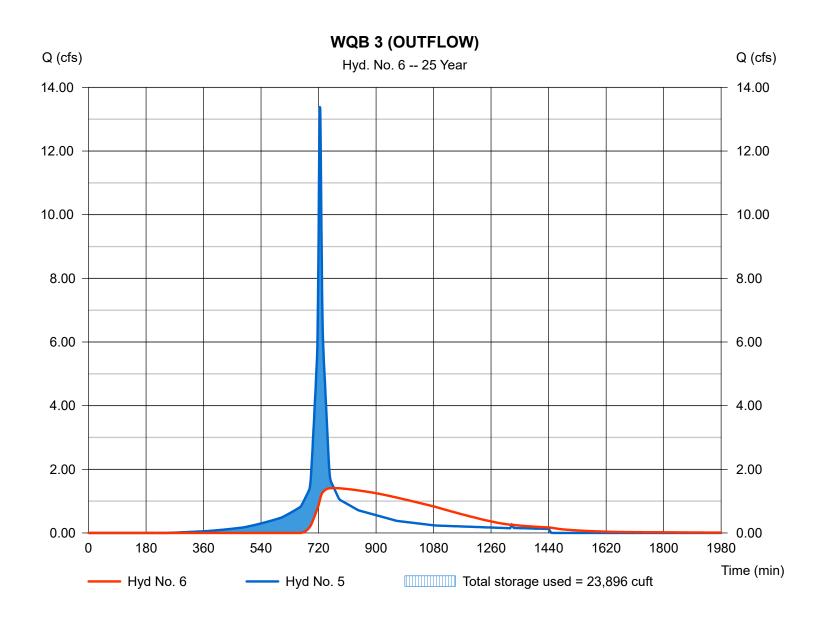
WQB 3 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 1.410 cfsStorm frequency Time to peak = 25 yrs = 768 min Time interval = 1 min Hyd. volume = 37,346 cuftInflow hyd. No. = 5 - WS-PR-S-1 TO WQB #3 Max. Elevation = 162.48 ft

Max. Storage

Storage Indication method used.

Reservoir name



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

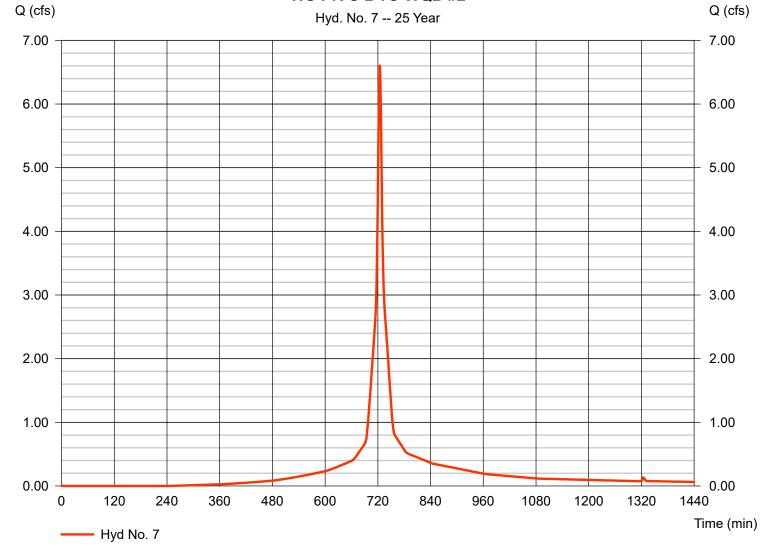
Hyd. No. 7

WS-PR-S-2 TO WQB #2

Hydrograph type = SCS Runoff Storm frequency = 25 yrs Time interval = 1 min Drainage area = 1.130 acBasin Slope = 0.0 % Tc method = USER Total precip. = 6.40 inStorm duration = 24 hrs

Peak discharge = 6.603 cfsTime to peak = 724 min Hyd. volume = 21,201 cuft Curve number = 88 Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Distribution = Type III Shape factor = 484

WS-PR-S-2 TO WQB #2



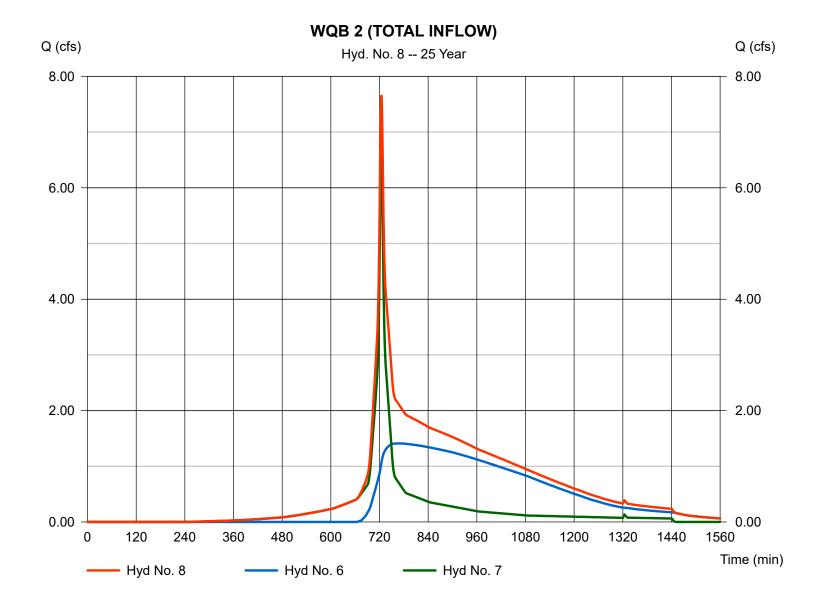
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 8

WQB 2 (TOTAL INFLOW)

Hydrograph type = Combine Storm frequency = 25 yrs Time interval = 1 min Inflow hyds. = 6, 7 Peak discharge = 7.653 cfs Time to peak = 725 min Hyd. volume = 58,547 cuft Contrib. drain. area = 1.130 ac



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

= 1.290 cfs

= 52,963 cuft

= 968 min

Hyd. No. 9

WQB 2 (OUTFLOW)

Hydrograph type = Reservoir
Storm frequency = 25 yrs
Time interval = 1 min

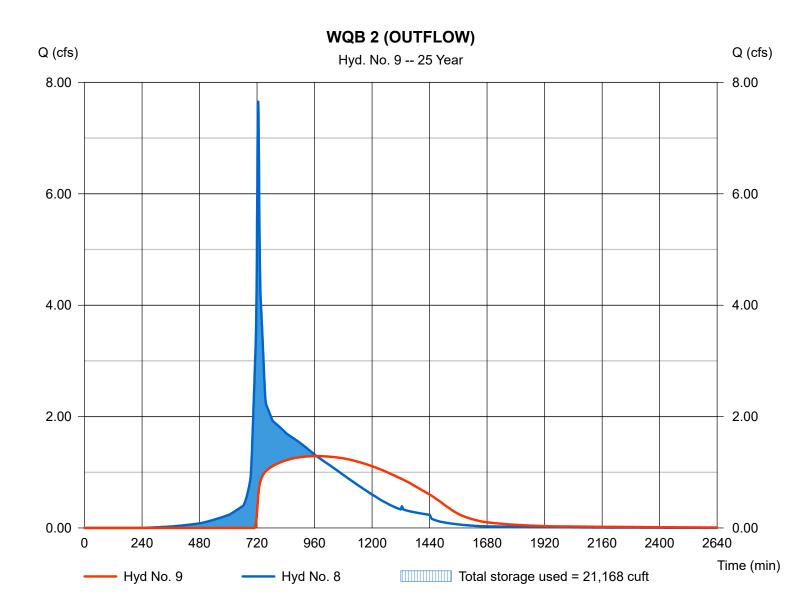
Inflow hyd. No. = 8 - WQB 2 (TOTAL INFLOW) Max. Elevation = 161.11 ft
Reservoir name = WQB2 Max. Storage = 21,168 cuft

Peak discharge

Time to peak

Hyd. volume

Storage Indication method used.



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 10

WS-PR-S-3 TO WQB #1

Hydrograph type = SCS Runoff Storm frequency = 25 yrs Time interval = 1 min Drainage area = 4.820 acBasin Slope = 0.0 % Tc method = USER Total precip. = 6.40 inStorm duration = 24 hrs

Peak discharge = 24.98 cfs
Time to peak = 726 min
Hyd. volume = 89,646 cuft
Curve number = 89
Hydraulic length = 0 ft

Time of conc. (Tc) = 10.00 min
Distribution = Type III
Shape factor = 484

WS-PR-S-3 TO WQB #1 Q (cfs) Q (cfs) Hyd. No. 10 -- 25 Year 28.00 28.00 24.00 24.00 20.00 20.00 16.00 16.00 12.00 12.00 8.00 8.00 4.00 4.00 0.00 0.00 120 240 360 480 600 720 1080 1200 1320 1440 1560 0 840 960 Time (min) Hyd No. 10

Hydraflow Hydrographs by Intelisolve v9.1

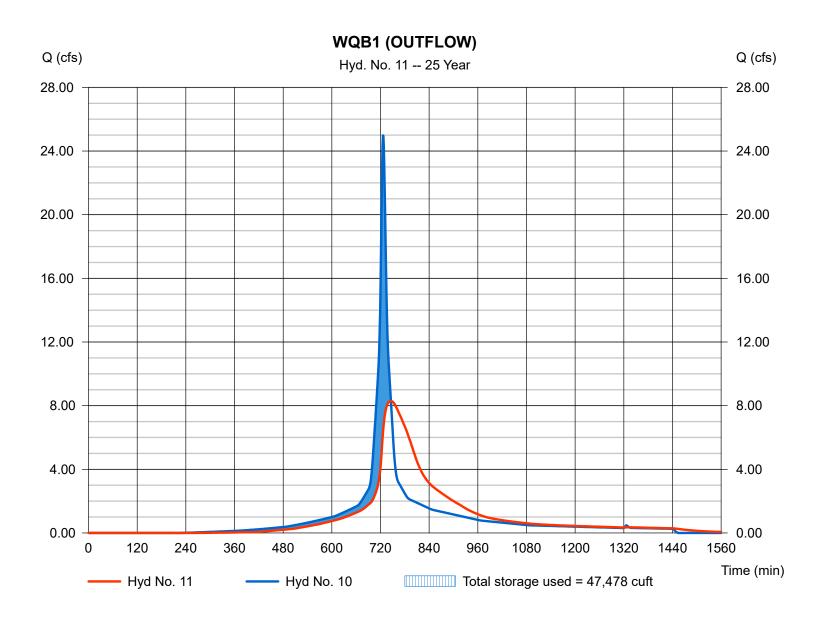
Friday, Dec 22, 2023

Hyd. No. 11

WQB1 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 8.290 cfsTime to peak Storm frequency = 25 yrs = 746 min Time interval = 1 min Hyd. volume = 89,633 cuft Inflow hyd. No. = 10 - WS-PR-S-3 TO WQB #1 Max. Elevation = 151.77 ft Reservoir name = WQB1 Max. Storage = 47,478 cuft

Storage Indication method used. Wet pond routing start elevation = 149.00 ft.



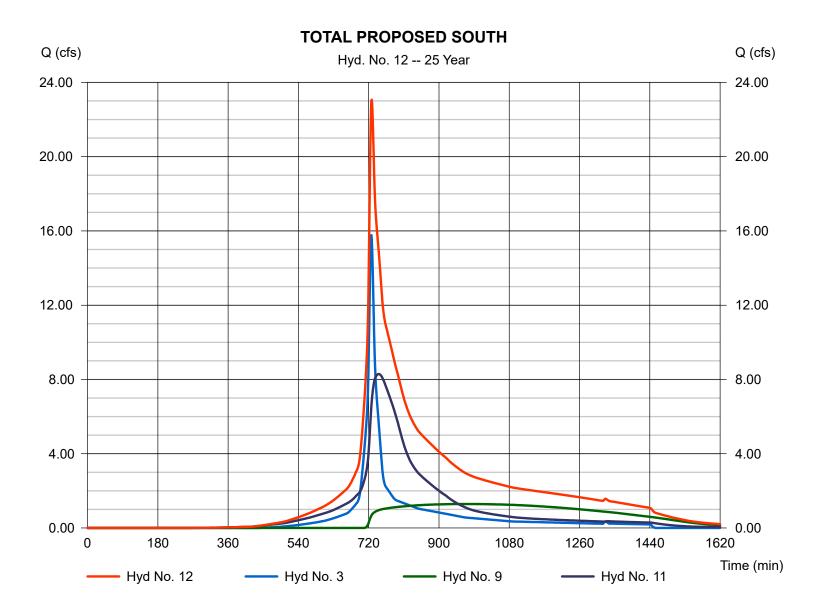
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 12

TOTAL PROPOSED SOUTH

Hydrograph type = Combine Storm frequency = 25 yrs Time interval = 1 min Inflow hyds. = 3, 9, 11 Peak discharge = 23.06 cfs
Time to peak = 728 min
Hyd. volume = 196,688 cuft
Contrib. drain. area = 3.890 ac



Hydrograph Summary Report

Hydraflow Hydrographs by Intelisolve v9.1

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	SCS Runoff	44.85	1	727	153,736				WS-EX-S
2	SCS Runoff	13.56	1	726	43,667				WS-EX-E
3	SCS Runoff	19.31	1	727	66,492				WS-PR-UNDET-S
4	SCS Runoff	2.442	1	725	7,580				WS-PR-UNDET-E (TOTAL PROP. EA
5	SCS Runoff	15.78	1	724	51,119				WS-PR-S-1 TO WQB #3
6	Reservoir	1.567	1	771	45,501	5	163.00	28,432	WQB 3 (OUTFLOW)
7	SCS Runoff	7.784	1	724	25,225				WS-PR-S-2 TO WQB #2
3	Combine	8.978	1	725	70,725	6, 7			WQB 2 (TOTAL INFLOW)
9	Reservoir	1.441	1	991	65,133	8	161.58	25,477	WQB 2 (OUTFLOW)
10	SCS Runoff	29.37	1	726	106,369				WS-PR-S-3 TO WQB #1
11	Reservoir	9.190	1	747	106,356	10	152.22	52,963	WQB1 (OUTFLOW)
12	Combine	27.62	1	727	237,981	3, 9, 11			TOTAL PROPOSED SOUTH
Mad	cro Model 202	23-12-22.	gpw		Return F	Period: 50 Y	'ear	Friday, Dec	22, 2023

Hydraflow Hydrographs by Intelisolve v9.1

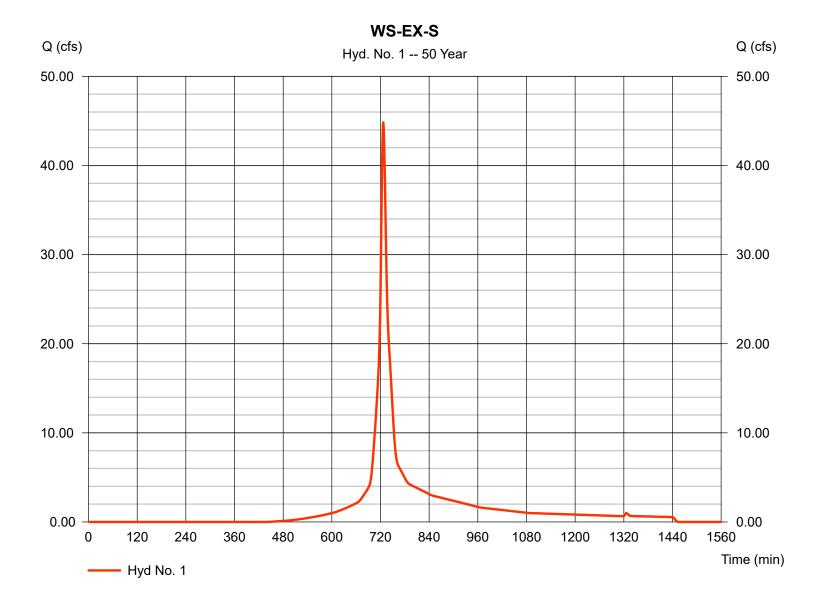
Friday, Dec 22, 2023

Hyd. No. 1

WS-EX-S

Hydrograph type = SCS Runoff Storm frequency = 50 yrsTime interval = 1 min Drainage area = 9.680 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 7.38 inStorm duration = 24 hrs

Peak discharge = 44.85 cfs= 727 min Time to peak Hyd. volume = 153,736 cuft Curve number = 74 Hydraulic length = 0 ftTime of conc. (Tc) $= 9.10 \, \text{min}$ Distribution = Type III = 484 Shape factor



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

= 13.56 cfs

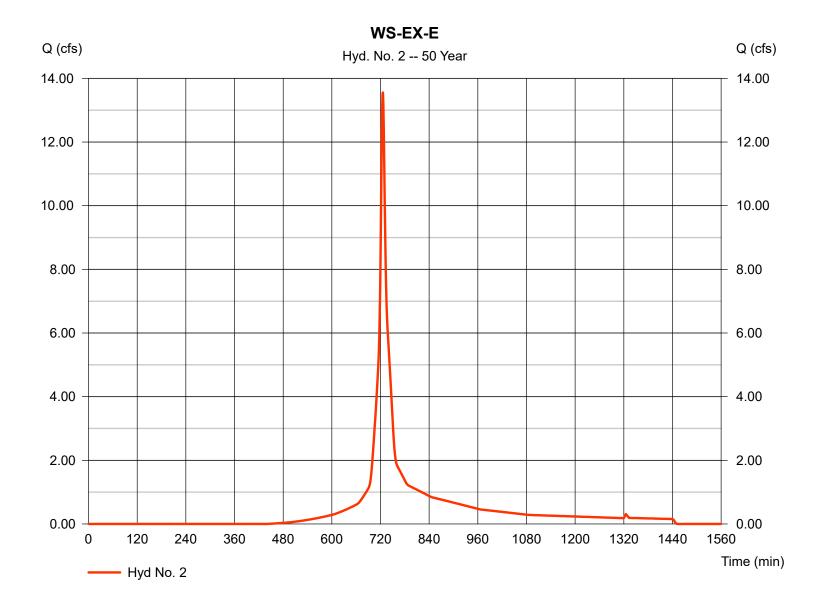
Hyd. No. 2

WS-EX-E

Hydrograph type = SCS Runoff Storm frequency = 50 yrsTime interval = 1 min Drainage area = 2.820 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 7.38 inStorm duration = 24 hrs

Time to peak = 726 min
Hyd. volume = 43,667 cuft
Curve number = 74
Hydraulic length = 0 ft
Time of conc. (Tc) = 7.50 min
Distribution = Type III
Shape factor = 484

Peak discharge



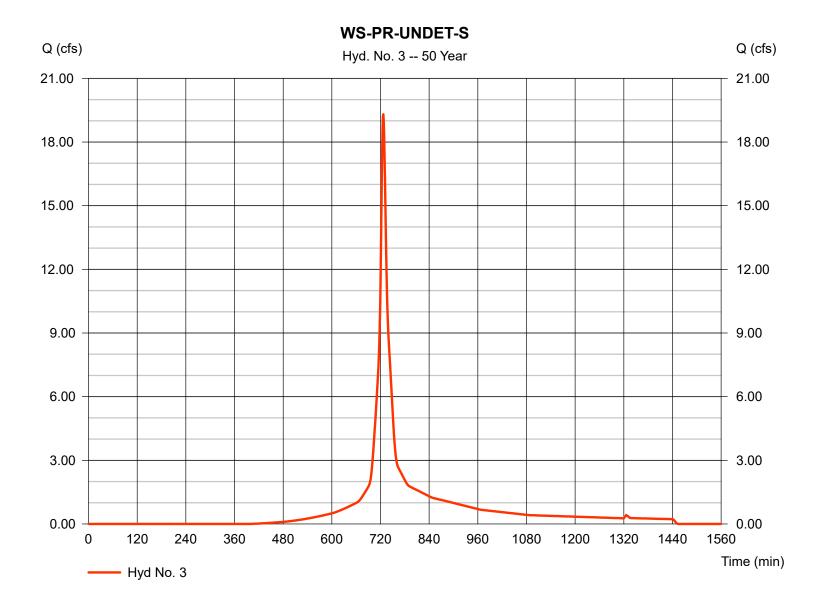
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 3

WS-PR-UNDET-S

Hydrograph type = SCS Runoff Peak discharge = 19.31 cfsStorm frequency Time to peak = 50 yrs= 727 min Time interval = 1 min Hyd. volume = 66,492 cuft Drainage area = 3.890 acCurve number = 77 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = USER Time of conc. (Tc) $= 10.00 \, \text{min}$ Distribution Total precip. = 7.38 in= Type III Storm duration = 484 = 24 hrs Shape factor



Hydraflow Hydrographs by Intelisolve v9.1

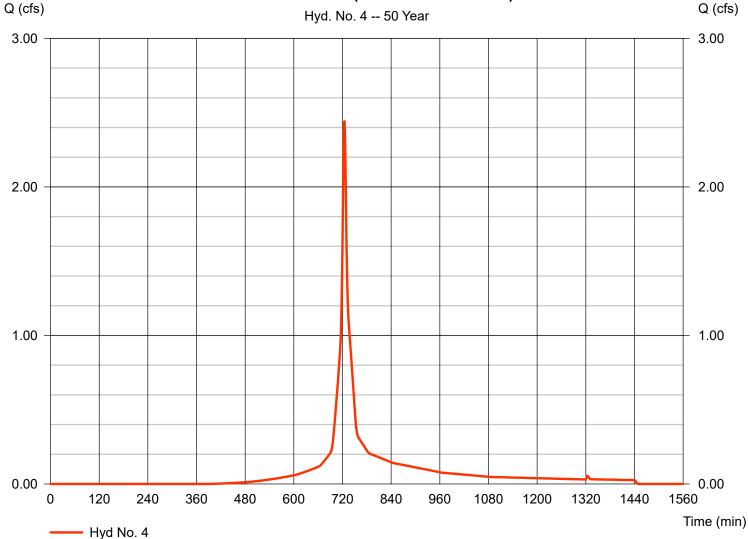
Friday, Dec 22, 2023

Hyd. No. 4

WS-PR-UNDET-E (TOTAL PROP. EAST)

= SCS Runoff Hydrograph type Peak discharge = 2.442 cfsStorm frequency Time to peak = 725 min = 50 yrsTime interval = 1 min Hyd. volume = 7,580 cuftDrainage area = 0.430 acCurve number = 77 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = USER Time of conc. (Tc) $= 5.00 \, \text{min}$ Total precip. = 7.38 inDistribution = Type III Storm duration = 24 hrs Shape factor = 484





Hydraflow Hydrographs by Intelisolve v9.1

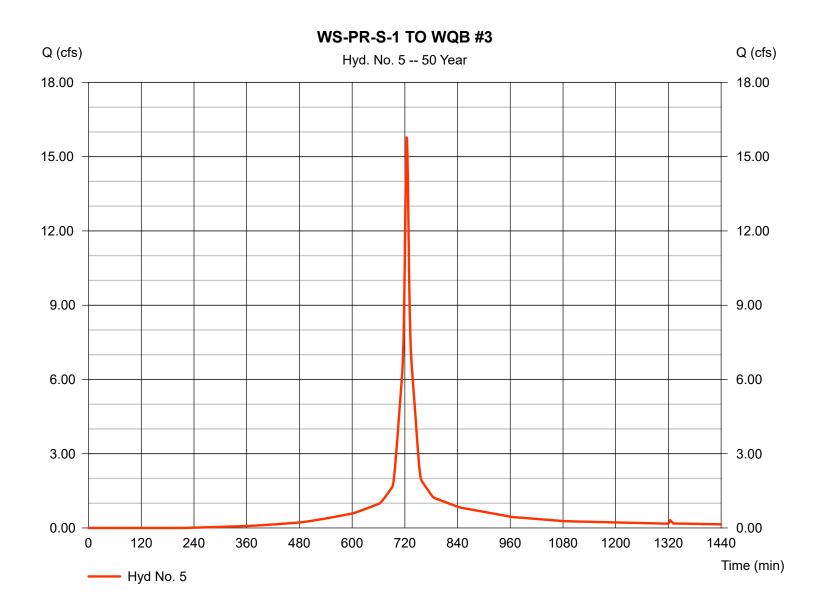
Friday, Dec 22, 2023

Hyd. No. 5

WS-PR-S-1 TO WQB #3

Hydrograph type = SCS Runoff Storm frequency = 50 yrsTime interval = 1 min Drainage area = 2.290 acBasin Slope = 0.0 % Tc method = USER Total precip. = 7.38 inStorm duration = 24 hrs

Peak discharge = 15.78 cfsTime to peak = 724 min Hyd. volume = 51,119 cuftCurve number = 88 Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Distribution = Type III = 484 Shape factor



= WQB3

Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

= 28,432 cuft

Hyd. No. 6

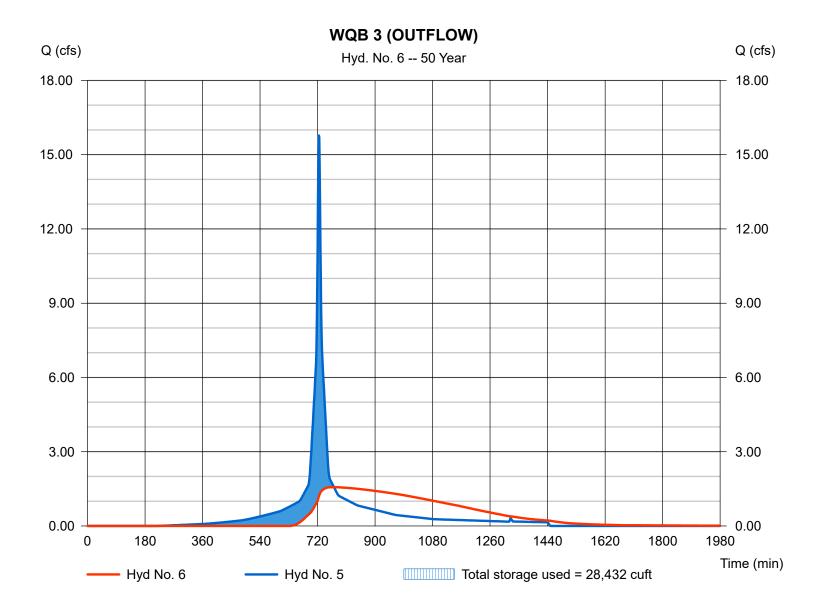
WQB 3 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 1.567 cfsStorm frequency Time to peak = 50 yrs= 771 min Time interval = 1 min Hyd. volume = 45,501 cuftInflow hyd. No. = 5 - WS-PR-S-1 TO WQB #3 Max. Elevation = 163.00 ft

Max. Storage

Storage Indication method used.

Reservoir name



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

= 7.784 cfs

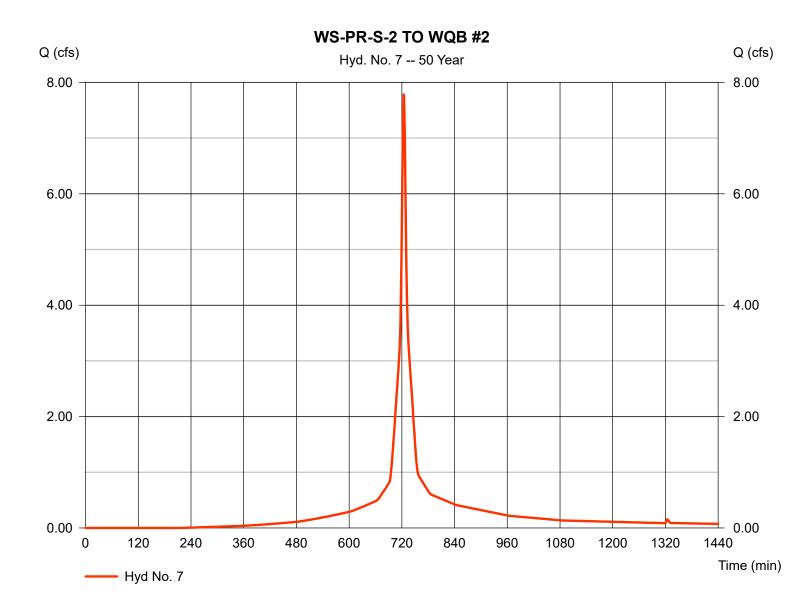
Hyd. No. 7

WS-PR-S-2 TO WQB #2

= SCS Runoff Hydrograph type Storm frequency = 50 yrsTime interval = 1 min Drainage area = 1.130 acBasin Slope = 0.0 % Tc method = USER Total precip. = 7.38 inStorm duration = 24 hrs

Time to peak = 724 min
Hyd. volume = 25,225 cuft
Curve number = 88
Hydraulic length = 0 ft
Time of conc. (Tc) = 5.00 min
Distribution = Type III
Shape factor = 484

Peak discharge



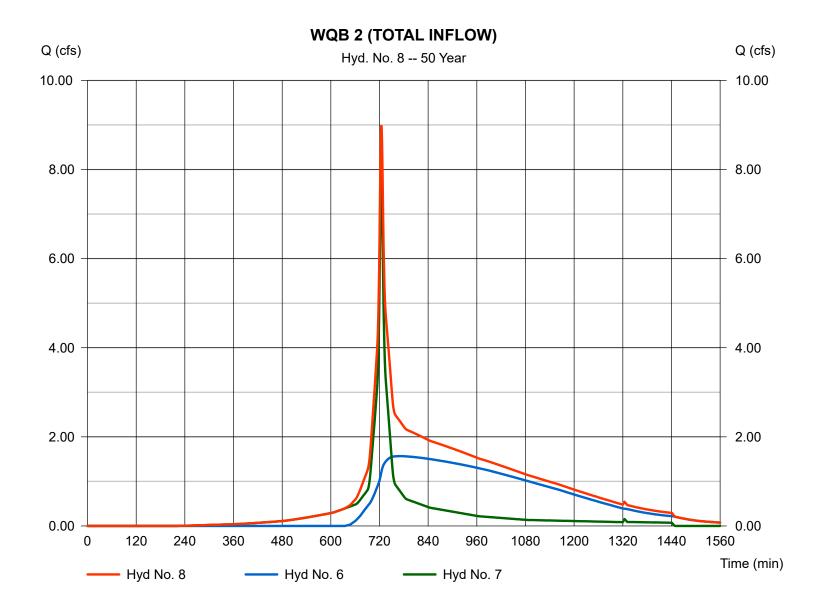
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 8

WQB 2 (TOTAL INFLOW)

Hydrograph type = Combine Storm frequency = 50 yrs Time interval = 1 min Inflow hyds. = 6, 7 Peak discharge = 8.978 cfs
Time to peak = 725 min
Hyd. volume = 70,725 cuft
Contrib. drain. area = 1.130 ac



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

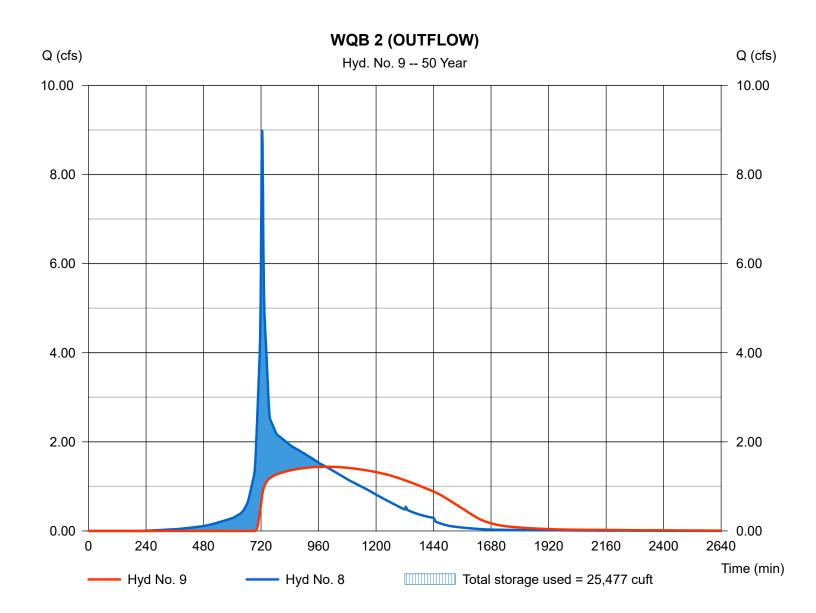
Hyd. No. 9

WQB 2 (OUTFLOW)

Hydrograph type= ReservoirPeak discharge= 1.441 cfsStorm frequency= 50 yrsTime to peak= 991 minTime interval= 1 minHyd. volume= 65,133 cuftInflow hyd. No.= 8 - WQB 2 (TOTAL INFLOW)Max. Elevation= 161.58 ft

Inflow hyd. No. = 8 - WQB 2 (TOTAL INFLOW) Max. Elevation = 161.58 ft
Reservoir name = WQB2 Max. Storage = 25,477 cuft

Storage Indication method used.



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 10

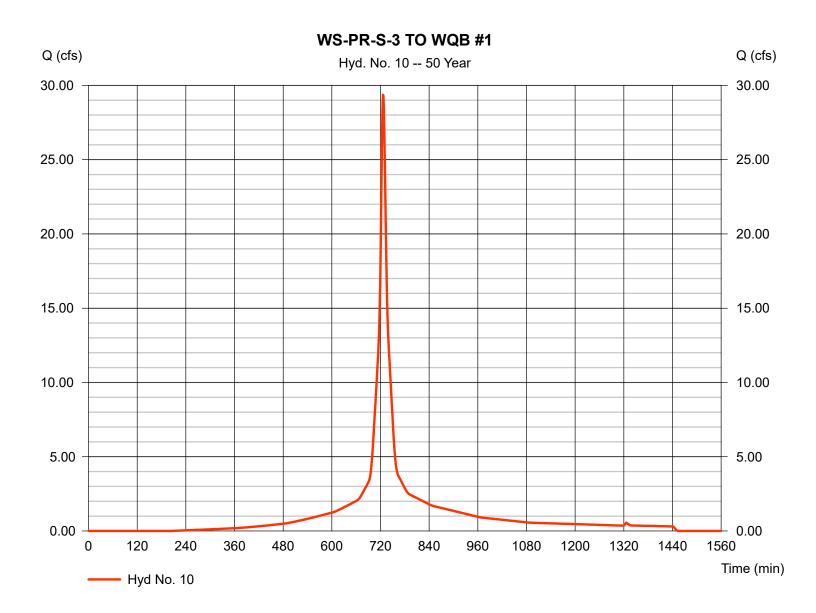
WS-PR-S-3 TO WQB #1

Hydrograph type = SCS Runoff Storm frequency = 50 yrsTime interval = 1 min Drainage area = 4.820 acBasin Slope = 0.0 % Tc method = USER Total precip. = 7.38 inStorm duration = 24 hrs

Peak discharge = 29.37 cfs
Time to peak = 726 min
Hyd. volume = 106,369 cuft
Curve number = 89

Hydraulic length = 0 ft
Time of conc. (Tc) = 10.00 min

Distribution = Type III Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.1

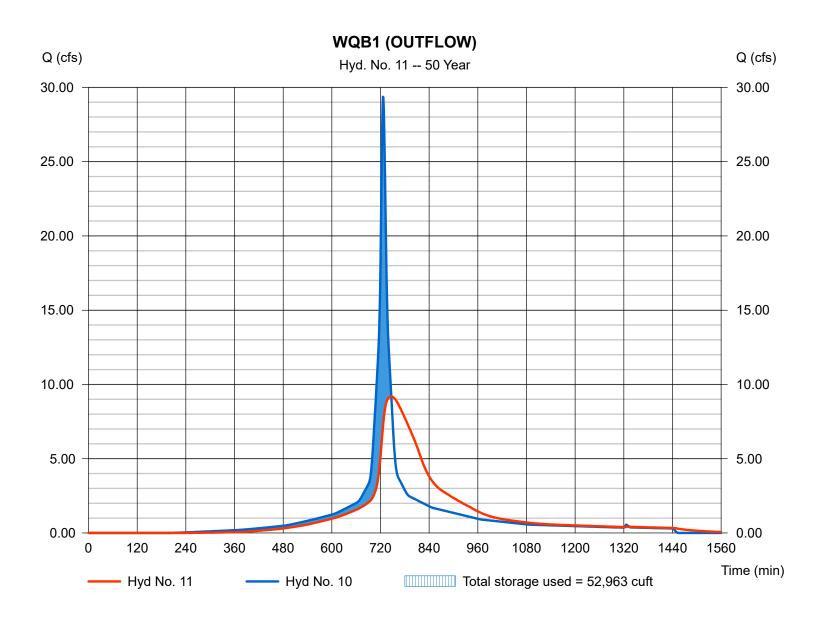
Friday, Dec 22, 2023

Hyd. No. 11

WQB1 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 9.190 cfsStorm frequency Time to peak = 50 yrs= 747 min Time interval = 1 min Hyd. volume = 106,356 cuft Inflow hyd. No. = 10 - WS-PR-S-3 TO WQB #1 Max. Elevation = 152.22 ftReservoir name = WQB1 Max. Storage = 52,963 cuft

Storage Indication method used. Wet pond routing start elevation = 149.00 ft.



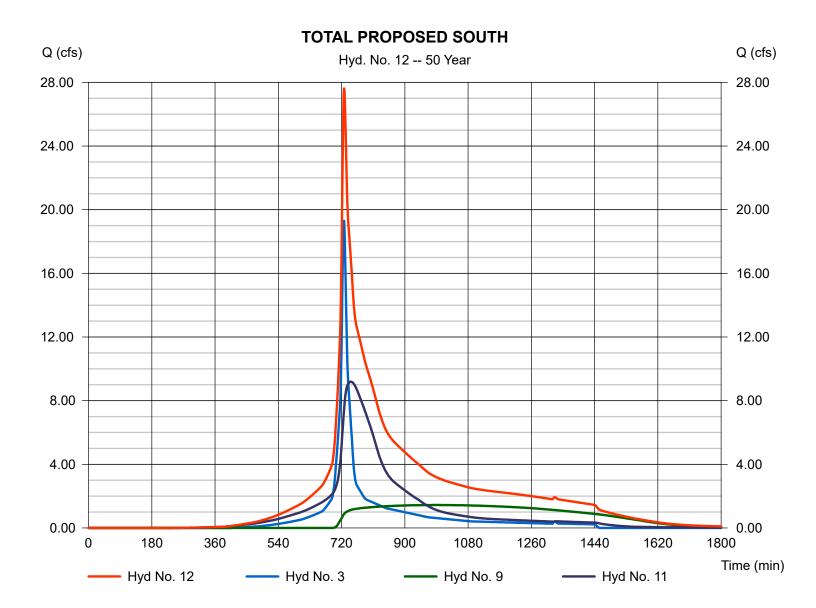
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 12

TOTAL PROPOSED SOUTH

Hydrograph type = Combine Storm frequency = 50 yrs Time interval = 1 min Inflow hyds. = 3, 9, 11 Peak discharge = 27.62 cfs
Time to peak = 727 min
Hyd. volume = 237,981 cuft
Contrib. drain. area = 3.890 ac



Hydrograph Summary Report

Hydraflow Hydrographs by Intelisolve v9.1

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	SCS Runoff	52.82	1	727	181,590				WS-EX-S
2	SCS Runoff	15.96	1	726	51,579				WS-EX-E
3	SCS Runoff	22.54	1	727	77,983				WS-PR-UNDET-S
4	SCS Runoff	2.849	1	725	8,890				WS-PR-UNDET-E (TOTAL PROP. EA
5	SCS Runoff	17.94	1	724	58,570				WS-PR-S-1 TO WQB #3
6	Reservoir	1.682	1	774	52,950	5	163.42	32,546	WQB 3 (OUTFLOW)
7	SCS Runoff	8.852	1	724	28,901				WS-PR-S-2 TO WQB #2
8	Combine	10.15	1	724	81,851	6, 7			WQB 2 (TOTAL INFLOW)
9	Reservoir	1.567	1	1006	76,249	8	162.00	29,400	WQB 2 (OUTFLOW)
10	SCS Runoff	33.33	1	726	121,634				WS-PR-S-3 TO WQB #1
11	Reservoir	9.891	1	748	121,621	10	152.62	58,169	WQB1 (OUTFLOW)
12	Combine	31.68	1	727	275,853	3, 9, 11			TOTAL PROPOSED SOUTH
Mad	cro Model 202	23-12-22.	gpw		Return P	eriod: 100	Year	Friday, Dec	22, 2023

Hydraflow Hydrographs by Intelisolve v9.1

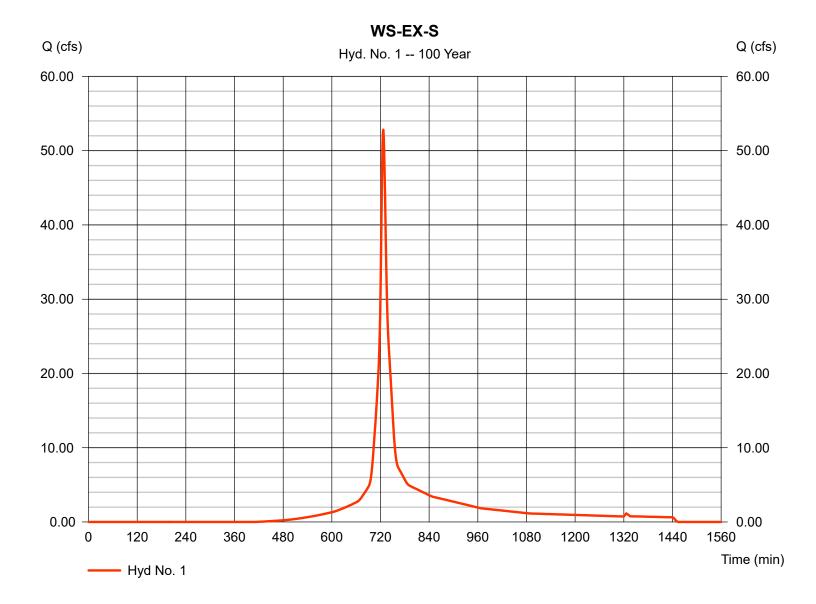
Friday, Dec 22, 2023

Hyd. No. 1

WS-EX-S

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval = 1 min Drainage area = 9.680 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 8.27 inStorm duration = 24 hrs

Peak discharge = 52.82 cfsTime to peak = 727 min Hyd. volume = 181,590 cuftCurve number = 74 Hydraulic length = 0 ftTime of conc. (Tc) $= 9.10 \, \text{min}$ Distribution = Type III = 484 Shape factor



Hydraflow Hydrographs by Intelisolve v9.1

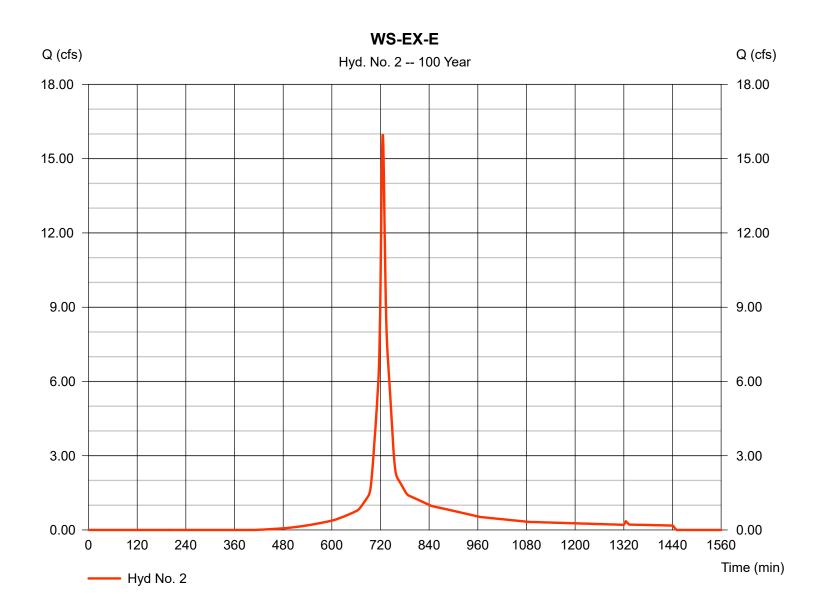
Friday, Dec 22, 2023

Hyd. No. 2

WS-EX-E

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval = 1 min Drainage area = 2.820 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 8.27 inStorm duration = 24 hrs

Peak discharge = 15.96 cfsTime to peak = 726 min Hyd. volume = 51,579 cuftCurve number = 74 Hydraulic length = 0 ftTime of conc. (Tc) $= 7.50 \, \text{min}$ Distribution = Type III = 484 Shape factor



Friday, Dec 22, 2023

Hydrograph Report

Hydraflow Hydrographs by Intelisolve v9.1

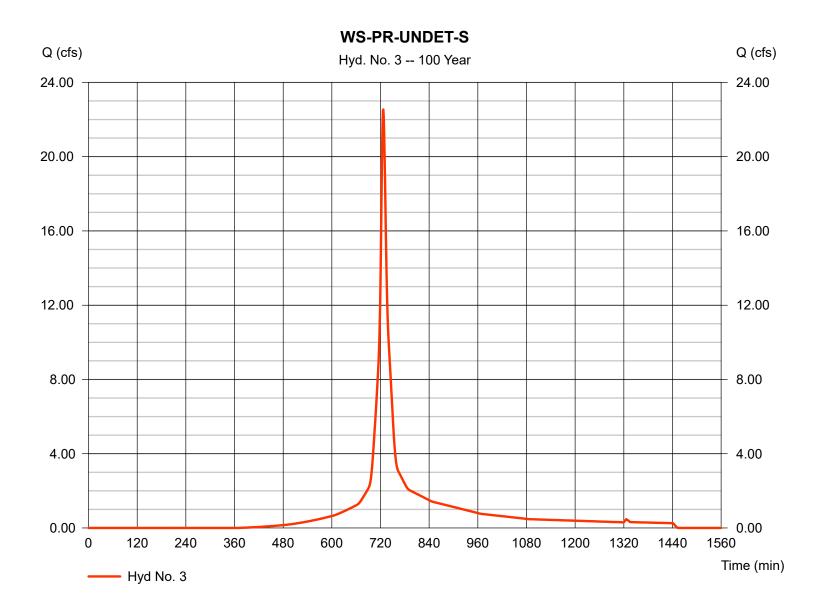
Hyd. No. 3

WS-PR-UNDET-S

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval = 1 min Drainage area = 3.890 acBasin Slope = 0.0 % Tc method = USER Total precip. = 8.27 inStorm duration = 24 hrs

Peak discharge = 22.54 cfs
Time to peak = 727 min
Hyd. volume = 77,983 cuft
Curve number = 77

Hydraulic length = 0 ft
Time of conc. (Tc) = 10.00 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.1

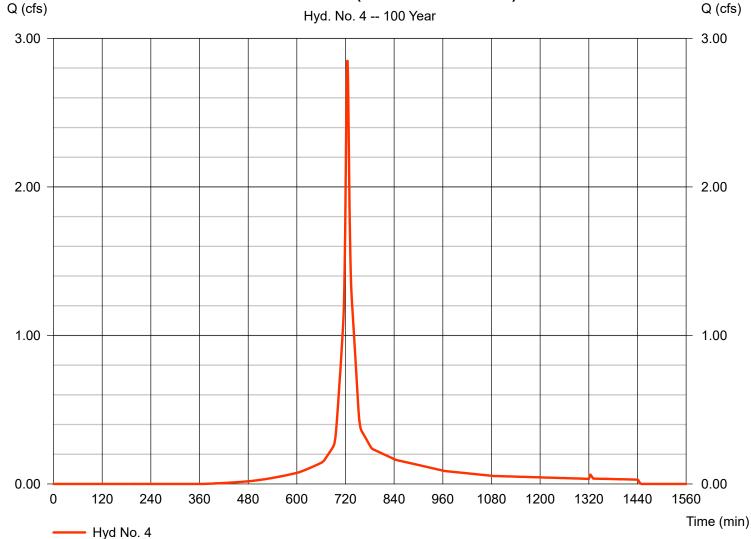
Friday, Dec 22, 2023

Hyd. No. 4

WS-PR-UNDET-E (TOTAL PROP. EAST)

= SCS Runoff Hydrograph type Peak discharge = 2.849 cfsStorm frequency Time to peak = 725 min = 100 yrsTime interval = 1 min Hyd. volume = 8,890 cuftDrainage area = 0.430 acCurve number = 77 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = USER Time of conc. (Tc) $= 5.00 \, \text{min}$ Distribution Total precip. = 8.27 in= Type III Storm duration = 24 hrs Shape factor = 484





Hydraflow Hydrographs by Intelisolve v9.1

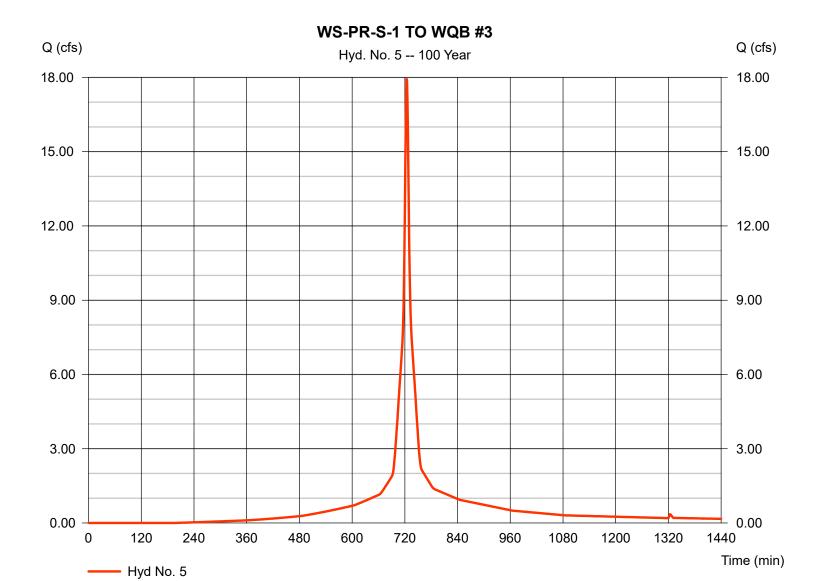
Friday, Dec 22, 2023

Hyd. No. 5

WS-PR-S-1 TO WQB #3

= SCS Runoff Hydrograph type Storm frequency = 100 yrsTime interval = 1 min Drainage area = 2.290 acBasin Slope = 0.0 % Tc method = USER Total precip. = 8.27 inStorm duration = 24 hrs

Peak discharge = 17.94 cfsTime to peak = 724 min Hyd. volume = 58,570 cuftCurve number = 88 Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Distribution = Type III = 484 Shape factor



= WQB3

Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

= 32,546 cuft

Hyd. No. 6

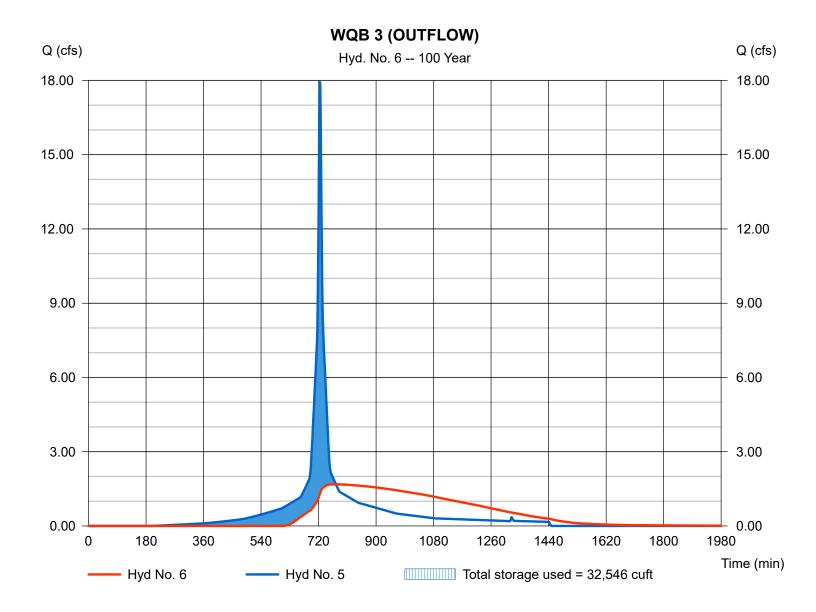
WQB 3 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 1.682 cfsStorm frequency Time to peak = 774 min = 100 yrsTime interval = 1 min Hyd. volume = 52,950 cuftInflow hyd. No. = 5 - WS-PR-S-1 TO WQB #3 Max. Elevation = 163.42 ft

Max. Storage

Storage Indication method used.

Reservoir name



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

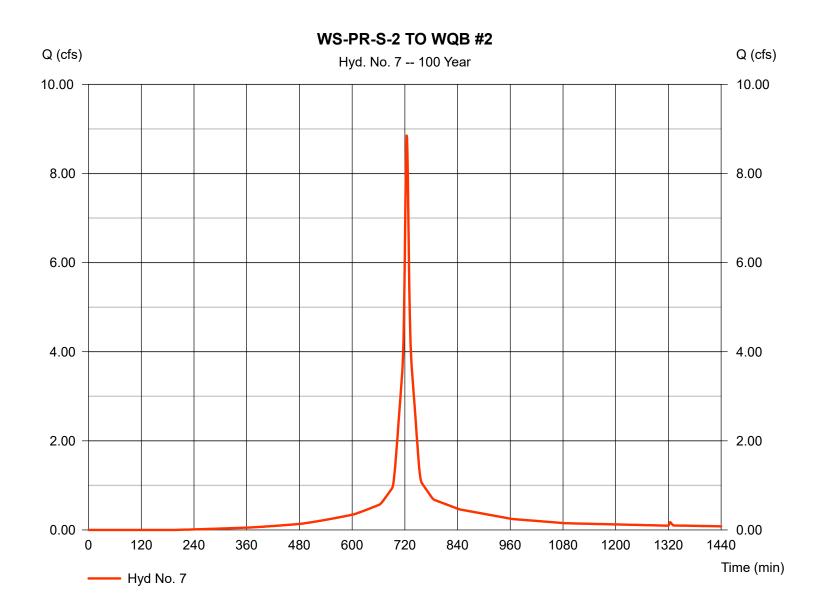
= 8.852 cfs

Hyd. No. 7

WS-PR-S-2 TO WQB #2

= SCS Runoff Hydrograph type Storm frequency = 100 yrsTime interval = 1 min Drainage area = 1.130 acBasin Slope = 0.0 % Tc method = USER Total precip. = 8.27 inStorm duration = 24 hrs

Peak discharge Time to peak = 724 min Hyd. volume = 28,901 cuft Curve number = 88 Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Distribution = Type III = 484 Shape factor



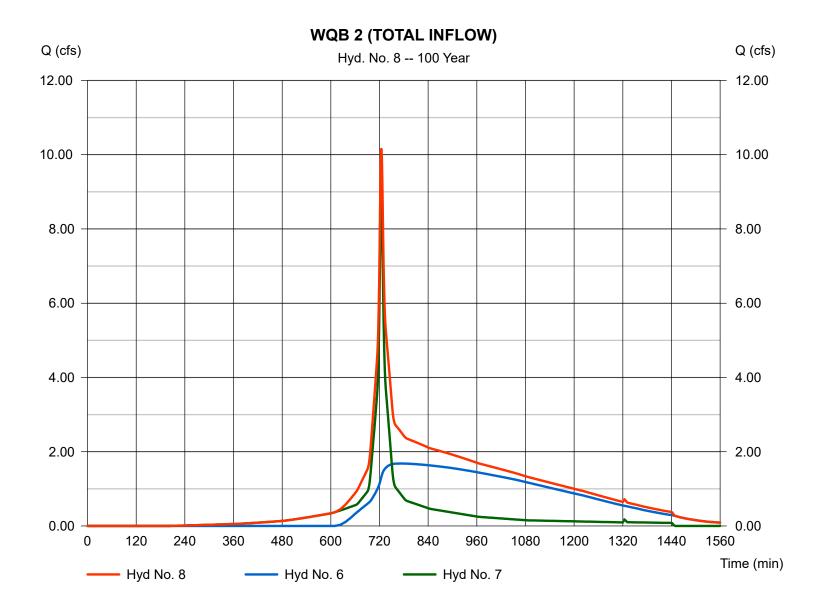
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 8

WQB 2 (TOTAL INFLOW)

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 1 min Inflow hyds. = 6, 7 Peak discharge = 10.15 cfs
Time to peak = 724 min
Hyd. volume = 81,851 cuft
Contrib. drain. area = 1.130 ac



= WQB2

Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

= 29,400 cuft

Hyd. No. 9

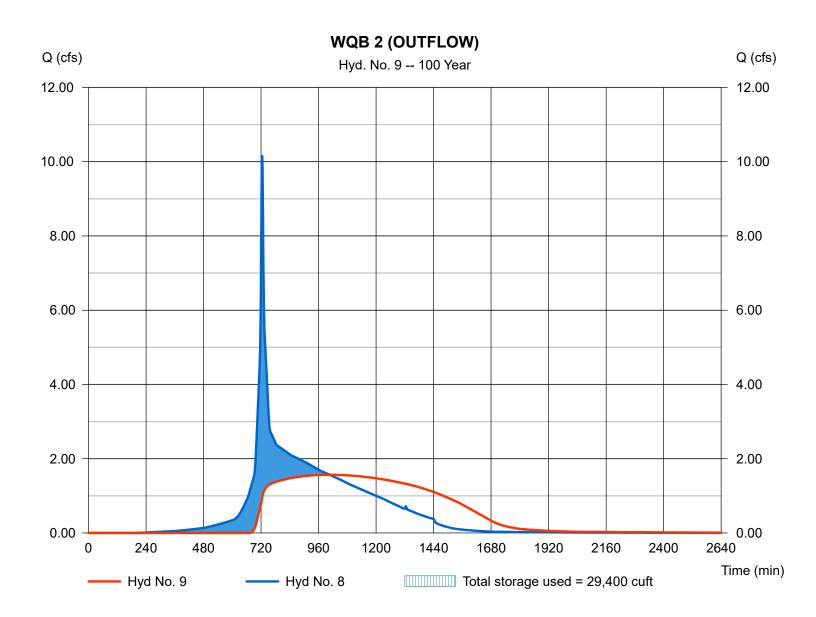
WQB 2 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 1.567 cfsStorm frequency Time to peak = 100 yrs $= 1006 \, \text{min}$ Time interval = 1 min Hyd. volume = 76,249 cuftInflow hyd. No. = 8 - WQB 2 (TOTAL INFLOW) Max. Elevation $= 162.00 \, \text{ft}$

Max. Storage

Storage Indication method used.

Reservoir name



Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 10

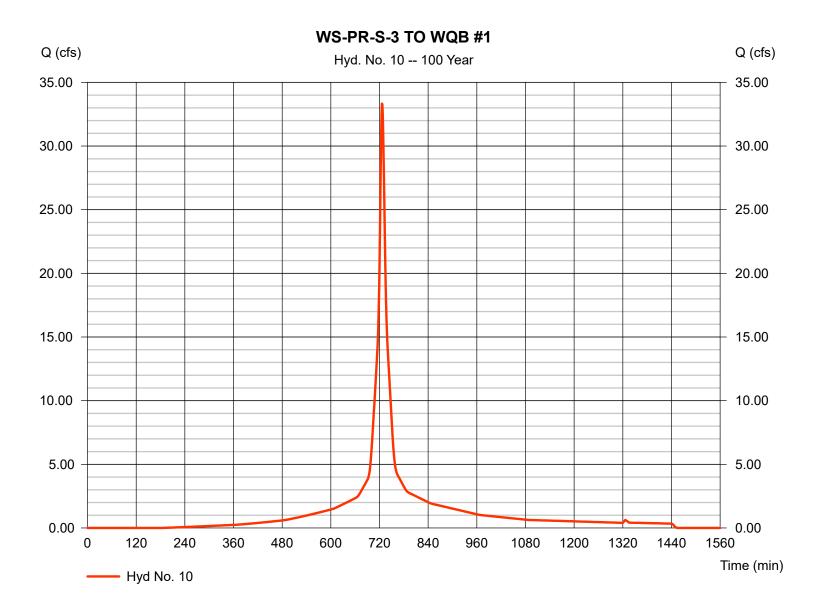
WS-PR-S-3 TO WQB #1

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval = 1 min Drainage area = 4.820 acBasin Slope = 0.0 % Tc method = USER Total precip. = 8.27 inStorm duration = 24 hrs

Peak discharge = 33.33 cfs
Time to peak = 726 min
Hyd. volume = 121,634 cuft
Curve number = 89
Hydraulic length = 0 ft
Time of conc. (Tc) = 10.00 min
Distribution = Type III

= 484

Shape factor



Hydraflow Hydrographs by Intelisolve v9.1

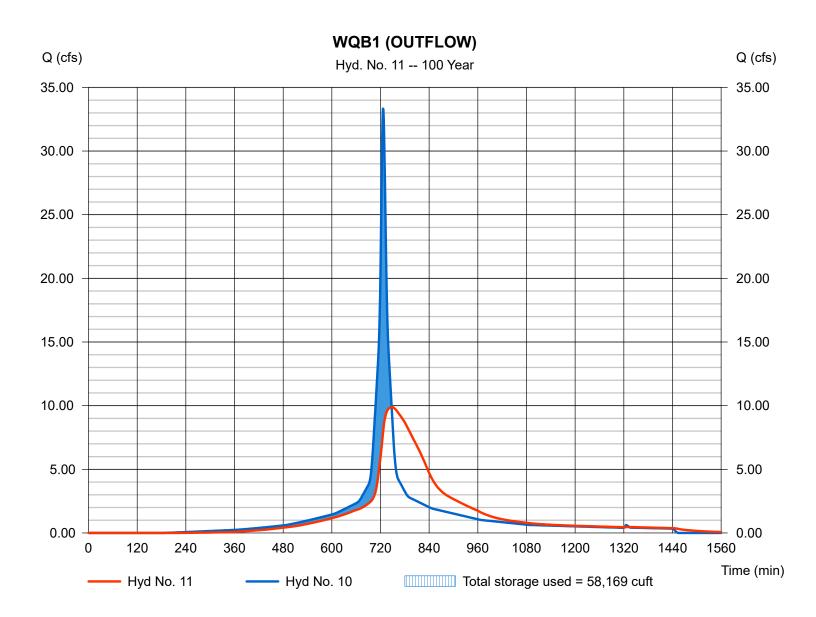
Friday, Dec 22, 2023

Hyd. No. 11

WQB1 (OUTFLOW)

Hydrograph type = Reservoir Peak discharge = 9.891 cfsStorm frequency = 100 yrsTime to peak = 748 min Time interval = 1 min Hyd. volume = 121,621 cuft Inflow hyd. No. = 10 - WS-PR-S-3 TO WQB #1 Max. Elevation = 152.62 ftReservoir name = WQB1 Max. Storage = 58,169 cuft

Storage Indication method used. Wet pond routing start elevation = 149.00 ft.



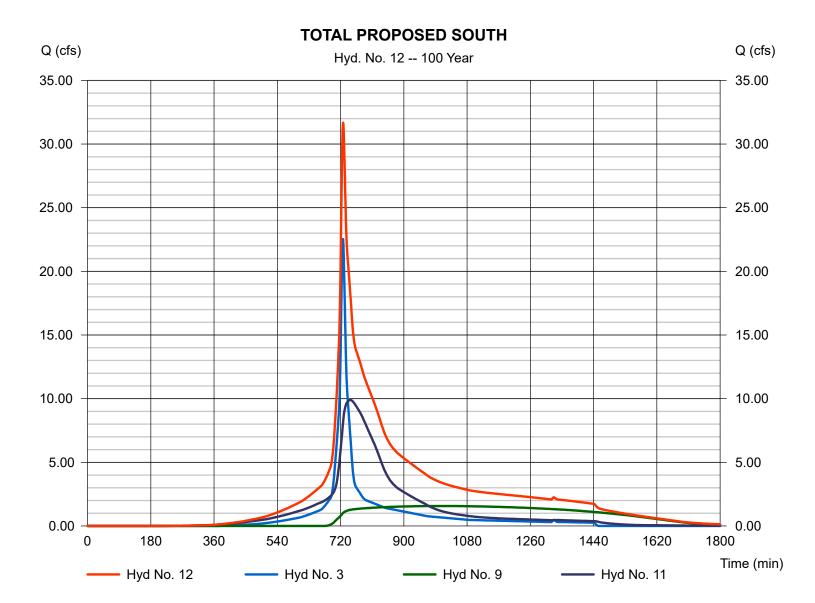
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Dec 22, 2023

Hyd. No. 12

TOTAL PROPOSED SOUTH

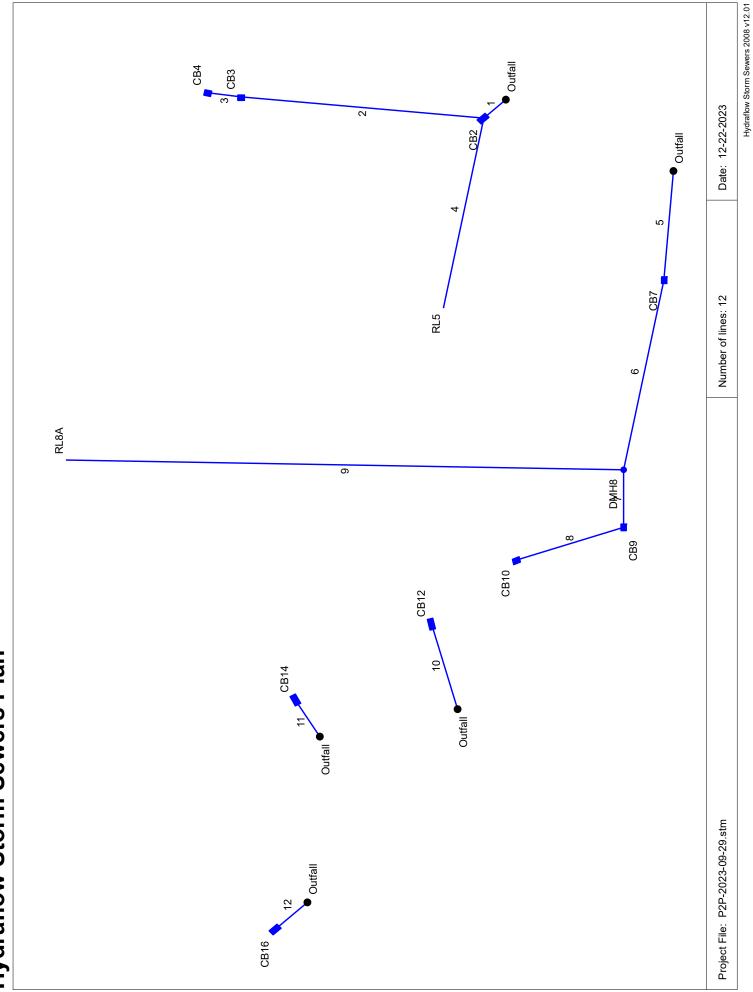
Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 1 min Inflow hyds. = 3, 9, 11 Peak discharge = 31.68 cfs
Time to peak = 727 min
Hyd. volume = 275,853 cuft
Contrib. drain. area = 3.890 ac



Attachment E

Pipe to Pipe Analysis

Hydraflow Storm Sewers Plan



Storm Sewer Summary Report

	f			•											
JZ	Line No.	Line ID	Flow rate (cfs)	Line size (in)	Line shape	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line slope (%)	HGL down (ft)	HGL up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns line No.	Junction Type
_		FES1-CB2	11.26	24	Öŗ	22	149.00	149.11	0.500	150.24	150.35	0.71	151.06	End	Combination
7		CB2-CB3	4.16	18	Ö	186	149.11	150.04	0.500	151.44*	151.69*	0.04	151.74	_	Combination
က		CB3-CB4	2.34	12	Ö	56	150.54	150.67	0.500	151.74*	151.83*	0.14	151.97	7	Combination
4		CB2-RL5	3.71	12	Oir	148	150.11	153.07	2.000	151.18	153.89	n/a	153.89 j	_	None
2		FES6-CB7	8.64	18	Cir	84	151.99	152.83	1.000	153.11	153.95	0.29	153.95	End	Combination
9		CB7-DMH8	8.18	18	Cir	148	152.83	153.94	0.750	154.20	155.03	n/a	155.03 j	2	Manhole
7		DMH8-CB9	5.50	15	Oir	44	154.19	154.94	1.705	155.27	155.88	n/a	155.88 j	9	Combination
∞		CB9-CB10	4.29	15	Ċ	98	154.94	155.49	0.640	156.17	156.40	0.31	156.71	7	Combination
6		DMH8-RL8A	3.71	12	Ċi	426	154.44	158.69	0.998	155.26	159.51	0.45	159.51	9	None
-	10	FES11-CB12	5.31	18	Cir	89	159.14	159.48	0.500	160.08	160.42	0.32	160.74	End	Combination
=	_	FES13-CB14	5.45	15	Ċi	34	162.00	162.51	1.500	162.93	163.44	n/a	163.44	End	Combination
-	12	FES15-CB16	6.70	15	Öİ	32	162.00	162.48	1.500	163.04	163.52	0.59	163.52	End	Combination
	Proje	Project File: P2P-2023-09-29.stm								Number of lines: 12	lines: 12		Run D	Run Date: 12-22-2023	-2023
	L	**************************************			-	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1							_		

NOTES: Return period = 25 Yrs.; *Surcharged (HGL above crown).; j - Line contains hyd. jump.

Hydraflow Storm Sewers 2008 v12.01

Inlet Report

Byp	S S	O#	₩O	₩ O	₩O	_	ij O	₩O	₩O	ð	₩O	₩O	ð	;	
	Depr (in)	2.0	2.0	2.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	2.0		23
Inlet	Spread (ft)	9.11	4.78	4.11	00.00	1.69	00.00	2.11	9.11	00.00	7.44	7.78	9.78		Run Date: 12-22-2023
	Depth (ft)	0.44	0.31	0.29	0.00	0.16	0.00	0.23	0.44	0.00	0.39	0.40	0.46		ın Date:
	Spread (ft)	9.11	4.78	4.11	0.00	06.9	0.00	2.11	9.11	0.00	7.44	7.78	9.78		<u> </u>
	Depth (ft)	0.27	0.14	0.12	00.00	0.07	00.00	90.0	0.27	00.00	0.22	0.23	0.29		-
	_	0.000	0.000	0.000	0.000	0.012	0.000	0.000	0.000	0.000	0.000	0.000	0.000		2
Gutter	Sx (ft/ft)	0:030	0:030	0:030	0.000	0.010	0.000	0.030	0:030	0.000	0:030	0:030	0.030		of lines: `
	Sw (ft/ft)	0:030	0.030	0.030	0.000	0.010	0.000	0.030	0.030	0.000	0.030	0.030	0.030		Number of lines: 12
	y (ft)	2.00	2.00	2.00	0.00	2.00	0.00	2.00	2.00	0.00	2.00	2.00	2.00		
	So (ft/ft)	Sag	Sag	Sag	Sag	0:030	Sag	Sag	Sag	Sag	Sag	Sag	Sag		
	× (£)	1.35	1.35	1.35	0.00	1.35	0.00	1.35	1.35	0.00	1.35	1.35	1.35		
Grate Inlet	⊣ (£)	4.62	2.31	2.31	0.00	2.31	0.00	2.31	2.31	0.00	4.62	4.62	4.62	<u> </u>	
Ō	area (sqft)	6.26	3.13	3.13	0.00	0.00	0.00	3.13	3.13	00.00	6.26	6.26	6.26		
Curb Inlet	(£) ا	5.83	2.73	2.73	00.00	2.73	00.00	2.73	2.73	00.00	5.83	5.83	5.83		
Curb	Ħ (ji)	3.0	3.0	3.0	0.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	3.0		
Junc	type	Comb	Comb	Comb	None	Comb	Ξ	Comb	Comb	None	Comb	Comb	Comb		
σ	cfs)	00.00	00.00	00.00	3.71	0.21	00.00	00.00	00.00	3.71	00.00	00.00	0.00		
σį	capt (cfs)	6.29	2.52	2.34	00.00	0.42	00.00	1.66	4.29	00:00	5.31	5.45	6.70		
σ	carry (cfs)	0.21	00.00	00:00	00.00	00.00	00.00	00.00	00.00	00:00	00.00	00.00	0.00		
# 5	cfs)	6.08	2.52	2.34	3.71	0.63	0.00	1.66	4.29	3.71	5.31	5.45	6.70		29.stm
Inlet ID		CB2	CB3	CB4	RL5	CB7	DMH8	CB9	CB10	RL8A	CB12	CB14	CB16		Project File: P2P-2023-09-29.stm
Line	0	-	7	က	4	2	9	7	∞	6	10	7	12	Į.	Project
															-

NOTES: Inlet N-Values = 0.016; Intensity = 42.54 / (Inlet time + 3.80) ^ 0.72; Return period = 25 Yrs.; * Indicates Known Q added. All curb inlets are Horiz throat.

Storm Sewer Tabulation

Stat	Station	Len	Drng Area	Area	Rnoff	Area	Area x C	2		Rain	Total	Cap	Vel	Pipe	ā	Invert Elev	Elev	HGL Elev	Elev	Grnd / Rim Elev	im Elev	Line ID	
Line	<u> </u>		Incr	Total	Соеп	Incr	Total	Inlet	Syst		MOIL MOIL	Ē		Size	Slope	Du	пр	Du	dΩ	ď	dn		
		(ft)	(ac)	(ac)	(c)			(min)	(min)	(in/hr)	(cfs)	(cfs)	(ft/s)	(in)	(%)	(#)	(#)	(ft)	(#)	(£)	(#)		
~	End	22	0.78	2.18	0.87	0.68	1.73	5.0	10.0	6.5	11.26	15.99	5.52	24	0.50	149.00	149.11	150.24	150.35	151.00	153.87	FES1-CB2	
7	~	186	0.32	0.94	0.88	0.28	0.64	5.0	10.0	6.5	4.16	8.04	2.36	18	0.50	149.11	150.04	151.44	151.69	153.87	153.87	CB2-CB3	
က	2	26	0.62	0.62	0.58	0.36	0.36	10.0	10.0	6.5	2.34	2.73	2.97	12	0.50	150.54	150.67	151.74	151.83	153.87	153.87	CB3-CB4	
4	~	148	0.46	0.46	06:0	0.41	0.41	5.0	5.0	0.6	3.71	5.46	5.06	12	2.00	150.11	153.07	151.18	153.89	153.87	158.00	CB2-RL5	
2	End	84	0.10	1.96	0.70	0.07	1.33	5.0	10.0	6.5	8.64	10.50	60.9	18	1.00	151.99	152.83	153.11	153.95	154.00	156.85	FES6-CB7	
9	2	148	00.00	1.86	00:00	00:00	1.26	0.0	10.0	6.5	8.18	9.85	5.39	18	0.75	152.83	153.94	154.20	155.03	156.85	159.11	СВ7-DМН8	
7	9	44	0.24	1.40	0.77	0.18	0.85	5.0	10.0	6.5	5.50	9.13	5.22	15	1.70	154.19	154.94	155.27	155.88	159.11	158.19	DMH8-CB9	
œ	7	98	1.16	1.16	0.57	99.0	99.0	10.0	10.0	6.5	4.29	5.59	3.99	15	0.64	154.94	155.49	156.17	156.40	158.19	158.74	CB9-CB10	
o	9	426	0.46	0.46	06:0	0.41	0.41	5.0	5.0	0.6	3.71	3.85	5.40	12	1.00	154.44	158.69	155.26	159.51	159.11	162.00	DMH8-RL8A	
10	End	89	0.74	0.74	0.80	0.59	0.59	5.0	5.0	0.6	5.31	7.43	4.57	18	0.50	159.14	159.48	160.08	160.42	161.67	162.98	FES11-CB12	
7	End	34	92.0	92.0	0.80	0.61	0.61	5.0	5.0	0.6	5.45	7.91	5.54	15	1.50	162.00	162.51	162.93	163.44	164.00	167.28	FES13-CB14	
12	End	32	1.15	1.15	0.65	0.75	0.75	5.0	5.0	0.6	6.70	7.91	6.16	15	1.50	162.00	162.48	163.04	163.52	164.00	167.28	FES15-CB16	
																							l l
Proje	Project File:	P2P-20.	P2P-2023-09-29.stm	9.stm												Number	Number of lines: 12	61		Run Dat	Run Date: 12-22-2023	2023	

NOTES: Intensity = 42.54 / (Inlet time + 3.80) ^ 0.72; Return period = 25 Yrs. ; Pipe travel time suppressed. ; c = cir e = ellip b = box

Attachment F

WQV Calculations

RUSSELL ROAD East Granby, Connecticut Water Quality Volume Calculations

PROPOSED	Woods	Landscape	Paved/Roof	Total	Woods	Landscape	Landscape Paved/Roof	Total
Water Quality Basin	Area (S.F.)	Area (S.F.)	Area (S.F.) Area (S.F.)	Area (S.F.)	Area (S.F.)	Area (S.F.)	Area (S.F.) Area (Acre)	Area (Acre)
WS-EX-E	25,476	578,121	14,828	618,425	85.0	13.27	0.34	14.20
WS-PR-S-1	0	42,916	56,874	06,790	00'0	66'0	1.31	2.29
WS-PR-S-2	0	22,251	26,880	49,131	00'0	0.51	0.62	1.13
WS-PR-S-3	4,480	65,947	138,038	215,079	0.10	1.51	3.17	4.79

STORM DRAINAGE SYSTEM DESIGN COMPUTATION SHEET

JOB: 23145 - EAST RUSSELL ROAD
nc.
Associates,
જ
. Hesketh
Щ. Д

Civil & Traffic Engineers - Surveyors CALCULATED BY: DRT

Planners - Landscape Architects CHECKED BY:

CULATED BY: DRT DATE: DECEMBER 22, 2023

CHECKED BY: _____CHECKED BY: _____

EXISTING & PROPOSED CONDITIONS

COVER CONDITION	WC	WOODS	LANDS	LANDSCAPED	IMPER	IMPERVIOUS		TOTAL	
RUNOFF 'C'	CN	CN ₁ =86	CN ^z	CN ₂ =78	ော	C ₃ =98			
DRAINAGE AREA (Ac.)	A_1	(AxC) ₁	A_2	$(AxC)_2$	A_3	(AxC) ₃	Α	AxC	C
WS-EX-E	0.58	50.30	13.27	1035.20	0.34	33.36	14.20	1068.56	75.27
WS-PR-S-1	0.00	0.00	0.99	76.85	1.31	127.95	2.29	204.80	89.40
WS-PR-S-2	0.00	0.00	0.51	39.84	0.62	60.47	1.13	100.32	88.94
WS-PR-S-3	0.00	0.00	1.51	118.09	3.17	310.55	4.68	428.64	91.53

RUSSELL ROAD

East Granby, Connecticut

Water Quality Volume Size Calculations

December 22, 2023

Minimum-Recommended Water Quality Volume (WQV)

Watershed	Total Area (Ac) A	Impervious Area (Ac)	Impervious (%) I	Runoff (R)	Min. Rec. WQV (ac-ft)	Min. Rec. WQV Min. Rec. WQV (ac-ft) (Cu.Ft.)
WS-PR-S-1	2.29	1.31	57.0	0.5629	0.10747	4,681
WS-PR-S-2	1.13	0.62	54.7	0.5424	0.05098	2,221
WS-PR-S-3	4.79	3.17	66.2	0.6459	0.25761	11,221

$$WQV = \frac{(1")(R)(A)}{12}$$
 WQV

WQV = water quality volume (ac-ft)

R = volumetric runoff coefficient 0.05+0.009(I)

I = percent impervious coverA= Site area (acres)

Provided Water Quality Volume

Water Quality Basins

	Elevations	Alea	Avg. Area	Avg. Depth	Net. WQV	WQV below	Total Rec. WQV
	(Ft.)	(Sq. Ft.)	(Sq. Ft.)	(FT)	(Cu. Ft.)	Outlet (Cu. Ft.)	(Cu. Ft.)
	146	5,069					
			6,068	2.00	12,135		
	148	7,066					
			7,604	1.00	7,604		
	149	8,143				19,740	11,221
WQ BASIN 1			8,268	1.00	8,268		
	150	9,469					
			10,859	2.00	21,718		
	152	12,249					
			13,684	2.00	27,369		
	154	15,120					
	158	4,878					
			6,095	1.00	6,095		
	159	6,023				6,095	2,221
			7,995	1.00	7,995		
WQ BASIN 2	160	7,311					
			8,639	2.00	17,279		
	162	9,968					
			11,471	2.00	22,941		
	164	12,974					
	159	5,133					
			5,604	1.00	5,604		
	160	6,076				5,604	4,681
			7,096	2.00	14,192		
WQ BASIN 3	162	8,116					
			9,249	2.00	18,498		
	164	10,382					
			11,628	2.00	23,255		
	166	12,874					

Attachment G

Rip-Rap Level Spreader

Calculations

10 RUSSELL ROAD AKA J.E.T.

Level Spreader Design Calculations

					MINIMUM	Minimum Calculated	Design Minimums	INIMUMS
Outlet ID	*	* OO	1/2 Do	* ML	La	Μ	La	M
	(CFS)	(Ft)		(Ft)				
PR-S-2 (WQB#2)	1.29	1.25	0.625	69.0	11.6	8.4	12	6
PR-S-3 (WQB#1)	8.29	1.25	0.625	0.55	20.1	23.8	21	24

TW=Depth of flow

 $La = (1.7)^*Q/Do^*(3/2) + 8(Do)$

W=3(Do)+0.4(La)

For Tw>(0.5)Do

For Tw<(0.5)Do

W=3(Do)+(La)

* From Hydrastorm Storm Sewers 2008 Analysis

Attachment H

Sediment Forebay

Calculations



F. A. Hesketh & Associates, Inc. 6 Creamery Brook East Granby, CT 06026

JOB 22145- 15 RUSSELL ROAD ARA J.E.T OF _____1 SHEET NO._ CALCULATED BY TORT DATE 12-22-23 CHECKED BY DATE 17-77-23 SCALE NOT.S.

Civil & Traffic Engineers • Surveyors • Planners • Landscape Architects

SEDMENT FOREBAY CALCULATIONS
$A = -\frac{2}{\pi} L u(1-E) = 5.566 \times 7.75504$
TSHERE:
A=MINIMUM REGISTRED SISKFACE OF SEDIMENT FOREBAY (SA
Q=DISCHARGE FROM DRAIMAGE ATTEA (CFS) =%TIDY/86,400 SEC
TO = D. DODY FEET PER GELDNO PARTICLE SETTING YELDUTY
E = SEDIMENT REMOVAL EFFICIENCY (ASSIMED DA OTO OF OTO)
TOB!: A = - (11,727/86,400) LU (1-0.9) = -37.987 LU (0) => 0.0004 LU (1-0.9) = -0.0004 LU (0) => 7447.661 S.F.
TORT: A, = (2721/86,400) Lu (1-0,9) = -02571 Lu(.1) => 0.0004 = -64Lu(.1) = 147.98 S.F.
TOB 3: A3 = (4,681/86,400) AN(1-0.9) = .55418 AN(1) + 0.0004 = -135LN 2.1) = 311.8885F